

Full Scale Fire test on a Post Tensioned Slab

Reinforced concrete structures in fire: a review of current rules

By Dr. Fergal Kelly

Synopsis
This review of current design rules for the resistance of reinforced concrete structures examines many of the assumptions on which these rules are based and presents evidence suggesting that the rules may be inappropriate both for the assessment of the effects of spalling and the prevention of collapse. Initially it reviews the consequences of spalling on the performance of key concrete elements within a structure before describing effects which occur in complete structures that are not apparent from standard fire tests. This includes a review of fire incidents in real buildings, a compartment fire test carried out on the concrete frame at BRE, Cardington, and a comparison with numerical models of concrete in fire. Results are also presented from a recent furnace test on a restrained post-tensioned slab. It concludes with a call for a fundamental review of the basis of the current rules and a programme of testing of modern structural components.

Notation
A Concrete cross-sectional area
A_r Reinforcement cross-sectional area
f_{cu} Concrete cylinder strength
f_{yk} Reinforcement strength
N_{Ed} Axial load in the fire limit state
N_{Ed,fi} Load ratio
t_{FRD,100} Fire resistance period in accordance with BS 8110 Part 2
k₁ Experimental fire resistance period
k₂ Concrete load duration factor
ω Mechanical reinforcement ratio

Introduction
High strength and self-compacting concrete is not considered in this paper as the likelihood of spalling, and the necessary mitigation measures, are well documented.

Significance of the spalling of structural elements
Although the occurrence of concrete spalling is linked to moisture content, the assumption that there is a safe, critical, moisture content level below which spalling will not occur is difficult to justify. Historically, the concept dates from work by Meyer-Ottawa in the early 1970s¹ when the experimental work carried out on single specimens established the idea of a critical moisture content. Although the study included the effect of stress due to applied load, the effect of varying permeability could not be assessed. Recent simulation work on spalling² has clearly indicated that a combination of moisture content and permeability contributes to the occurrence of spalling.
Flat slabs and columns are considered to be the structural elements most susceptible to the effects of excessive spalling and it is mainly those elements which are considered in this paper. With regard to other structural elements, furnace tests carried out on cast-in-place waffle slabs by CIRIA³ indicated that loss of cover due to spalling may not be as critical on ribbed slabs. Beams, which are highly reinforced with a single bar at each of the corners in the soffit, are also less likely to be critically affected by spalling as any midspan bars in the bottom layer (and any subsequent layers) will be less affected by temperature rise even when spalling occurs. This can be demonstrated by numerical calculations on heat transfer⁴. These forms of construction may however be susceptible to other failure mechanisms which are discussed in this paper.

Continuous post-tensioned flat slabs
Van Herbecken and Van Damme⁵ reported the results from eight fire tests on continuous post-tensioned flat slabs with unbonded tendons. The slabs were 180mm thick, one way spanning, continuous over the supports and loaded on the

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Table 1: Fire test data on continuous flat slabs

Slab no	Internal span (m)	Centreline span (m)	Aggregate	cover (mm)	age (days)	f _{yk} (N/mm ²)	f _{cu} (N/mm ²)
P1	6	2	Granit	20	275	49	80
P2	6	2	Granit	30	280	40	120-180
P3	6	2	Granit	30	274	71	120-180
P4	6	1.8	Granit	22.5	88	36	120-180
P5	6	1.8	Limestone	22.5	109	109	120-180
P6	6	1.8	Limestone	22.5	126	224	240
P7	6	1.8	Granit	40	126	36	120-180
P8	6	1.8	Granit	40	104	103	120-180

NOTES:
Slab P1: Complete collapse at 40 minutes. No spalling to be seen by the authors before the figure of 40 in the results and no comment on complete collapse at 40 minutes.
Slab P2: Test terminated at 40 minutes owing to excessive rates of collapse.
Slab P3: Test collapsed at 71 minutes.
Slab P4: Excessive spalling, test terminated in view of collapse risk.
Slab P5: Large amounts of spalling, test terminated.
Slab P6: Test terminated owing to excessive rates of collapse.
Slab P7: Secondary reinforcement visible after test, test terminated at stipulated collapse.
Slab P8: Test terminated owing to excessive rates of collapse.
Damage was spotted under f_{cu} when the arrival level of the bottom of the slab of the test was close to the top of the slab. The aggregate of slab P8 meets the minimum aggregate content with the data being from BS 8110 Part 2.



- Background
 - Increasing workload with PT slabs
 - Concerns voiced by suppliers
- Historical research
 - Surveys, large scale tests, numerical models
- Fire test on PT Slab
 - Configuration, results
- Overview and Conclusions
 - Further Testing

Background

- Development of concrete cover
 - BRE Study 1936-46
 - 13-64 mm cover (CP114 1957)
 - Increased cover = Increased fire resistance
 - Increased cover = Increase in spalling
 - More spalling with continuity
 - Spalling at moisture content as low as 2.3%
- Spalling – cause & effect
 - Moisture content & permeability
 - Stress, Aggregate type, etc
 - Loss of section, exposed rebar
 - Flat slabs and columns most vulnerable

Changes in Modern Concretes

- Composition change
 - Early strength rates

- Permeability
 - Higher strength concrete
 - Lower permeability

- Structural configuration
 - PT Slabs
 - FE design

Table 5: Historical data on the ratio of 3 to 28 day strengths of concrete

Date	Ratio of 3 to 28 day strength
Postwar	0.406
1953	0.424
1957	0.441
1960	0.457
1965	0.459
1970	0.500
1975	0.500
1980	0.545
1985	0.554
1990	0.553
1992	0.554

Historical Research

- PT Slabs

- Herberhen and Van Damme tests
- Value of recent tests questionable (PT and Precast)

- Columns

- 15% loss of area = 34% loss of strength
- Modern concrete may be more susceptible

Table 1: Fire test data on continuous flat slabs

Slab no	inte spar	Table 4: A comparison of the levels of spalling in various test series			BS8110 (min)	
		Test series	No. of columns tested	No. with severe spalling		Percentage with severe spalling
P1					60	
P2					10-180	
P3		National Building Studies 12	18	4	22	10-180
P4					10-180	
P5		National Building Studies 18	31	4	13	10-180
P6					>240	
P7		Series C	12	7	58	10-180
P8		Series D	20	18	90	10-180

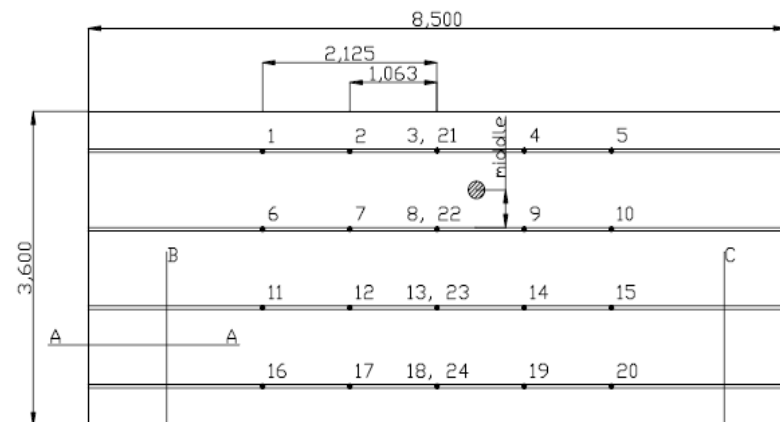
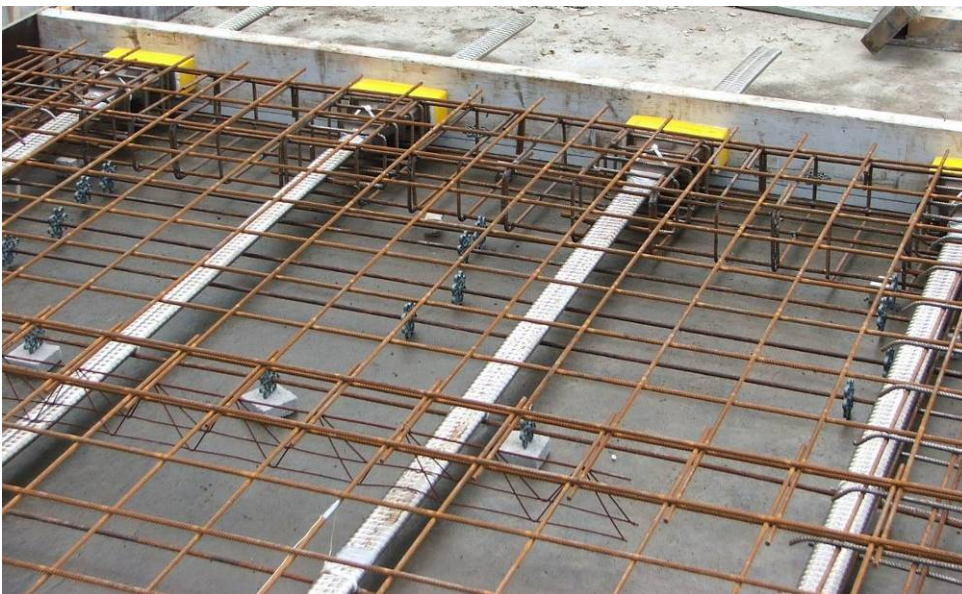
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Post tensioned slab fire test

■ Slab Configuration

- 8.5 x 3.6 m slab, 250 mm thick
- RC40 with OPC and Thames Valley Gravel, MC 4.6%
- Bonded tendons, 4 per duct, 40 cover
- 2 hour fire resistance
- 34 Type K thermocouples



B Thermocouples 21 t/m 24 on the side of the Tendon. The Thermocouples 1 to 20 on the bottomside of the Tendons.

● 10 thermocouples in the cage numbered CA1 up to CA 10

Post tensioned slab fire test

■ Test Configuration

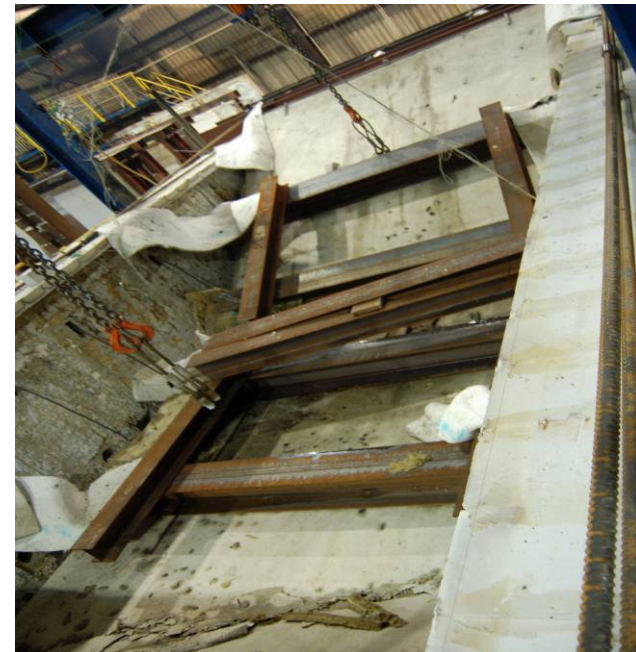
- Standard fire test temperature curve
- 20 type K thermocouples, plate (control) and wire
- Restrained test, 1200 kN force, 10% preset
- Twin hydraulic rams with spreader beams
- Deflection via 3 LVDT gauges at mid-span
- Load ratio of 0.5



Test Results

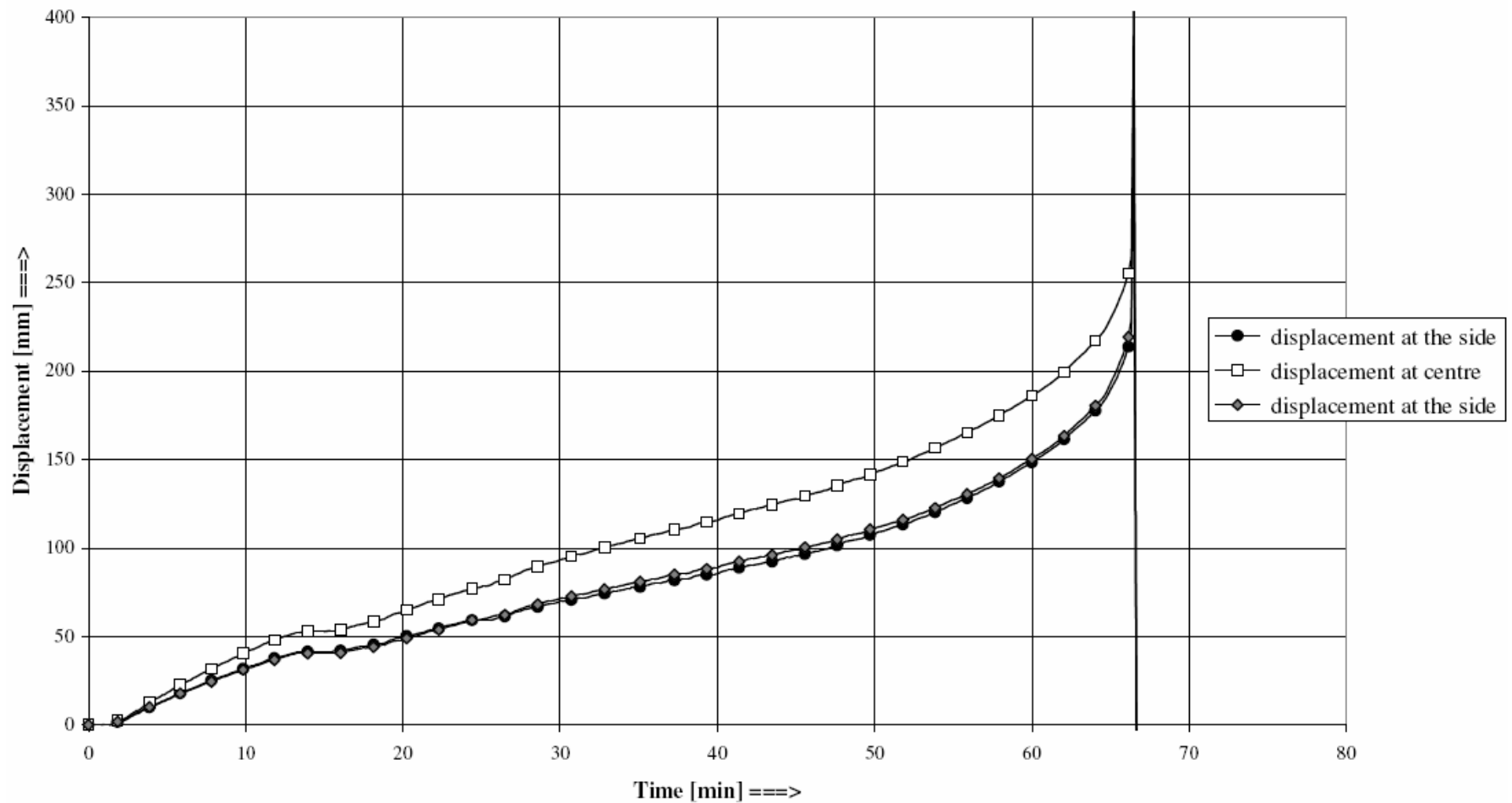
■ Summary

- 11 mins - Spalling began
- 18.5 mins - Max restraint force reached
- 20 mins - PT ducts visible
- 28 – Spalling reduces, steady increase in deflection
- 66 mins - Collapse



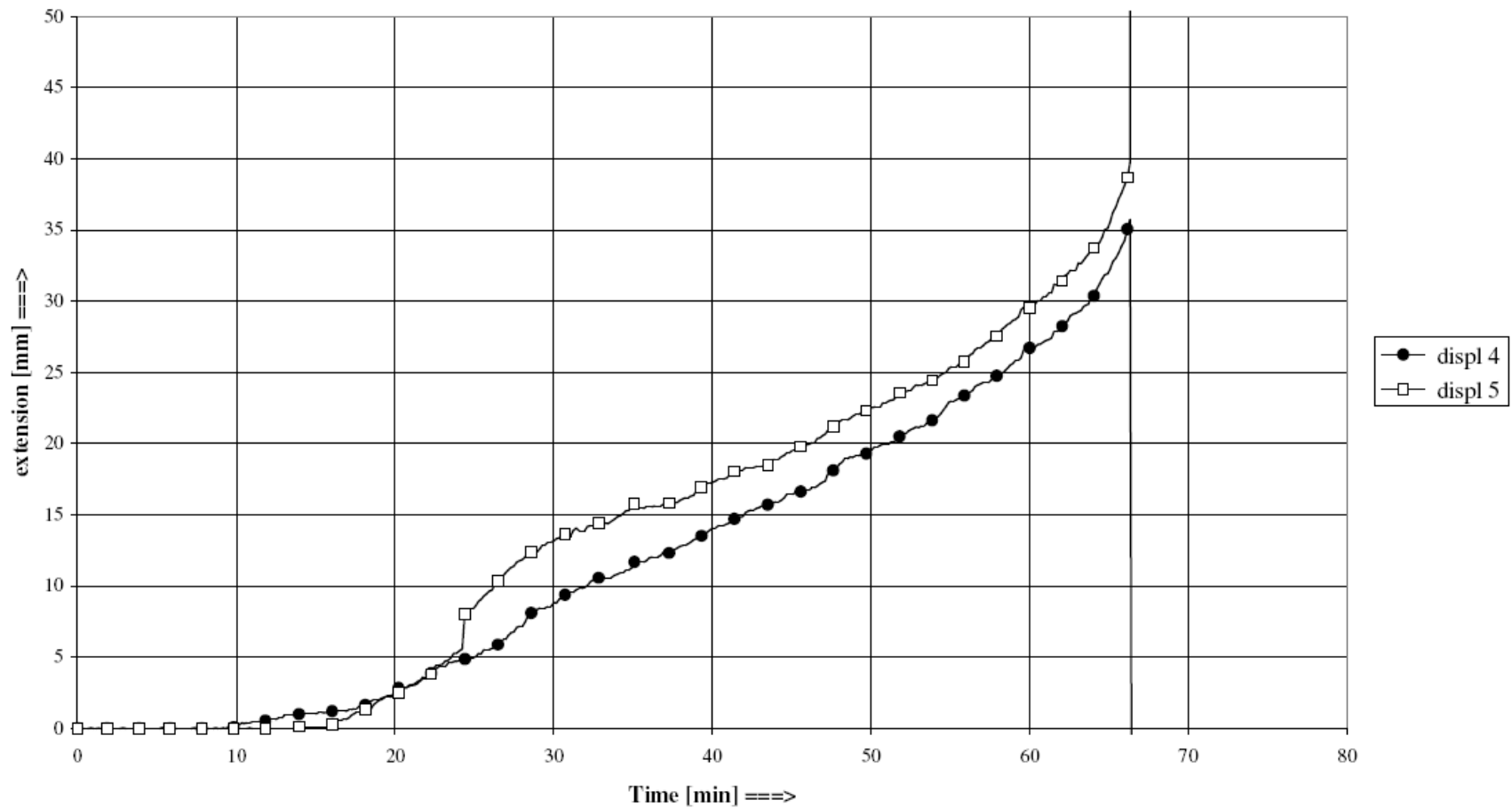
Test Results

■ Vertical Deflection



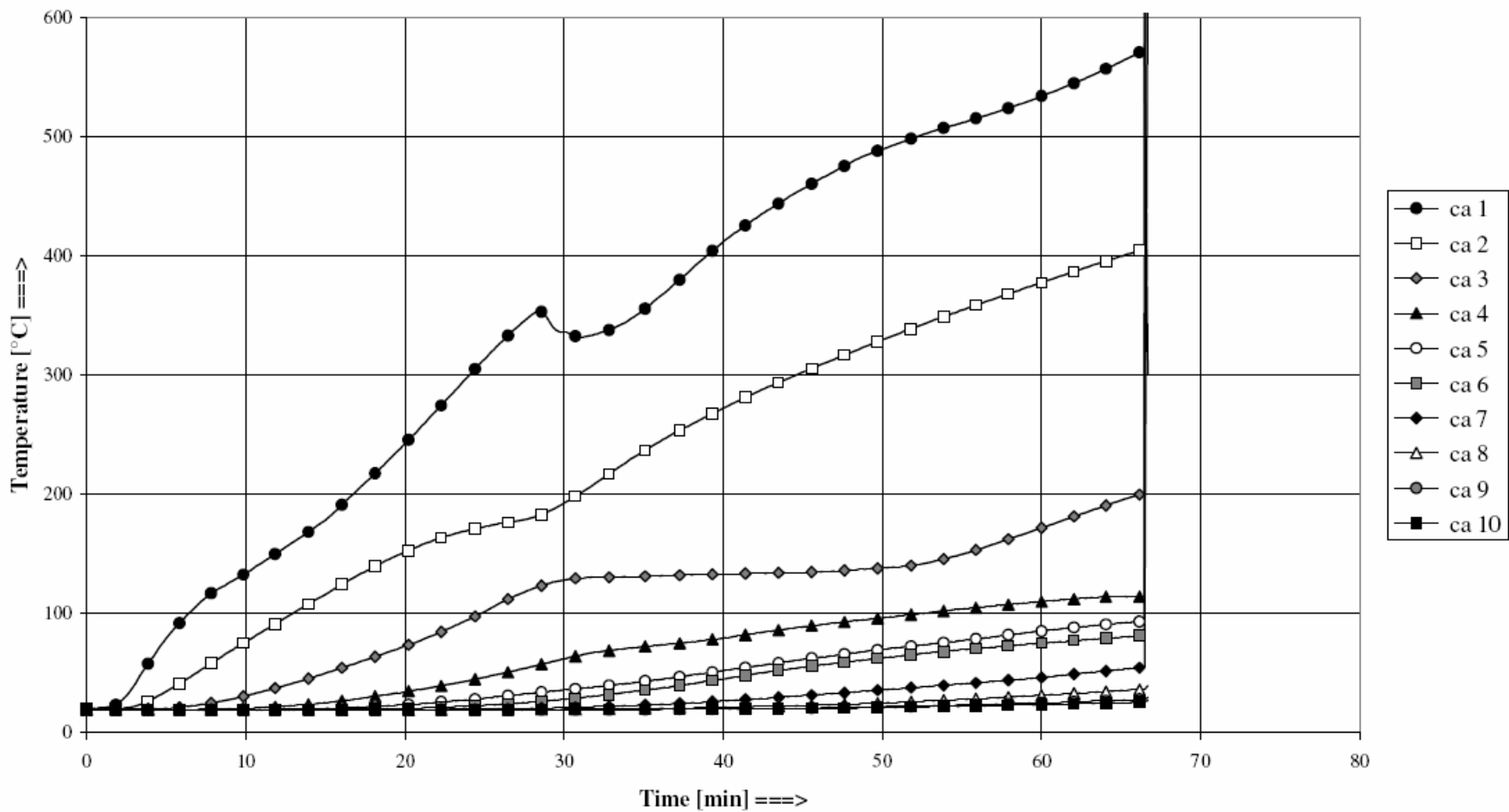
Test Results

■ Expansion



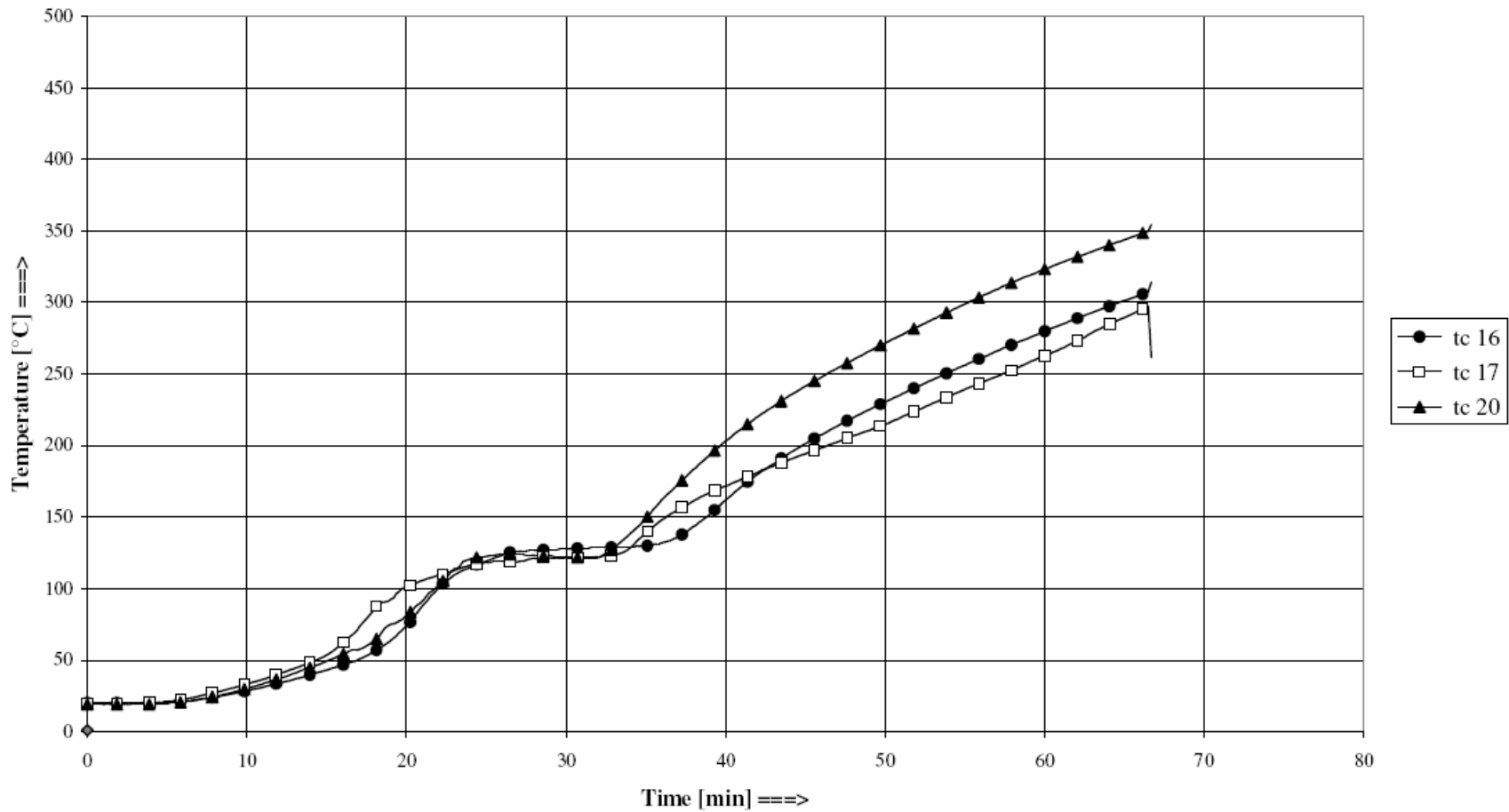
Test Results

■ Temperature Profile in Slab



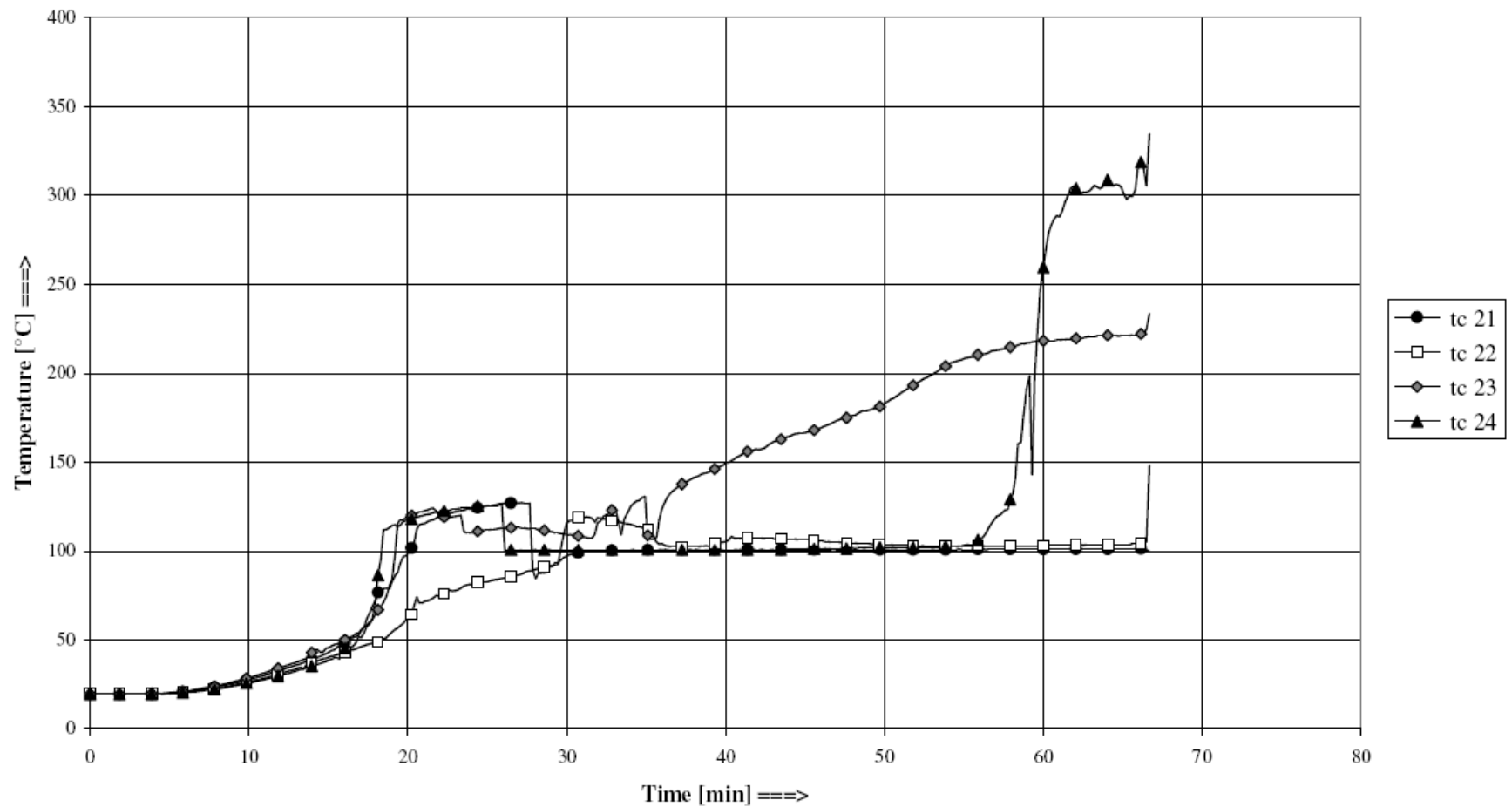
Test Results

- Temperature of PT Duct (Bottom)



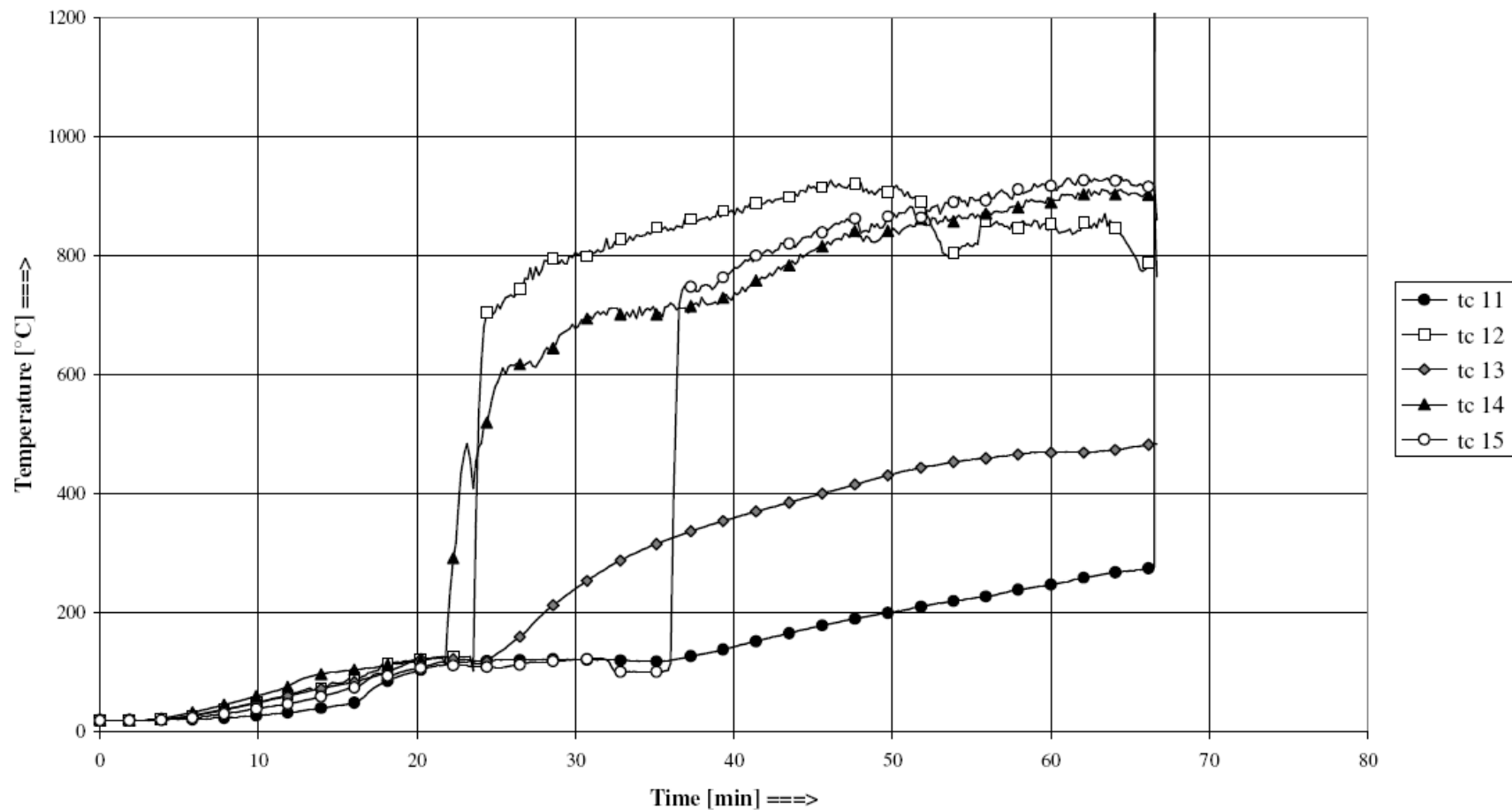
Test Results

■ Temperature of PT Duct (Side)



Test Results

- Temperature of PT Duct (Spalled)



- PT Slab Overview
 - PT stress +2 N/mm²
 - Restraint forces significant, ≥ 1.3 N/mm²
 - PT ducts vulnerable to spalling
 - Concrete cover questionable in terms of duct temperature even with minor spalling
 - Additional btm mesh should be considered
 - Additional large scale tests
 - Information on long-term moisture content

Questions