

# **Analysis of Catenary Action in Steel Beams under Fire Conditions**

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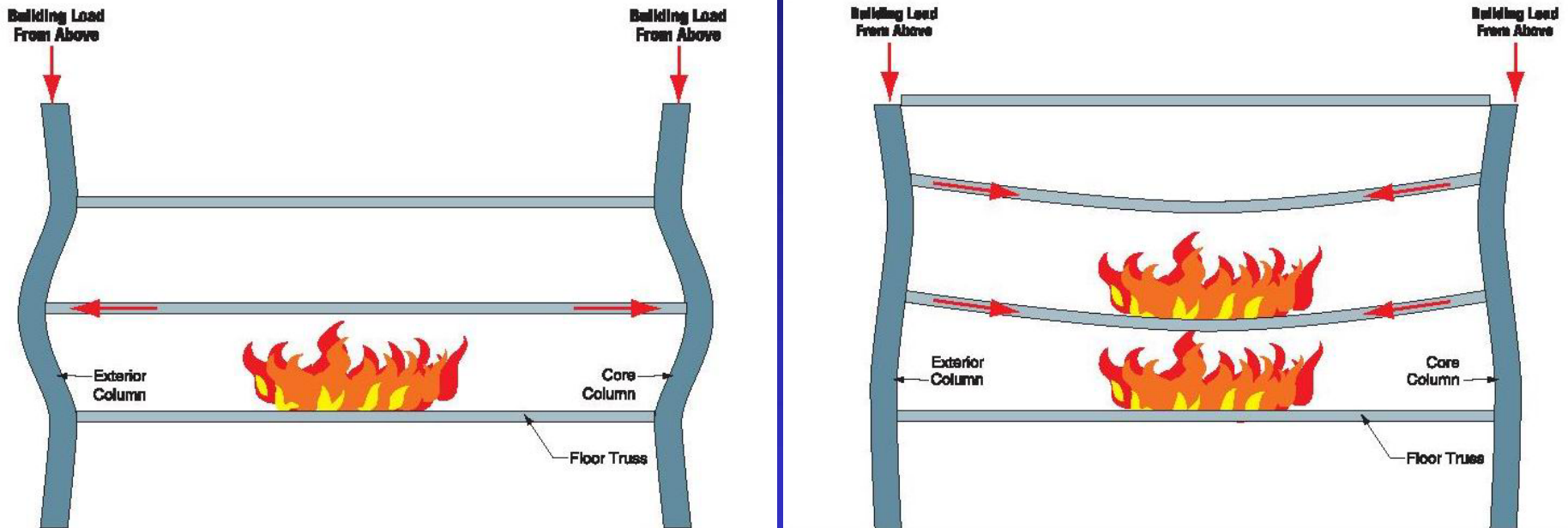
Manchester Centre for Civil and Construction Engineering

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# OVERVIEW

- Introduction
- Numerical Simulations by ABAQUS
- Hand Calculation Method
- Conclusions

# Introduction

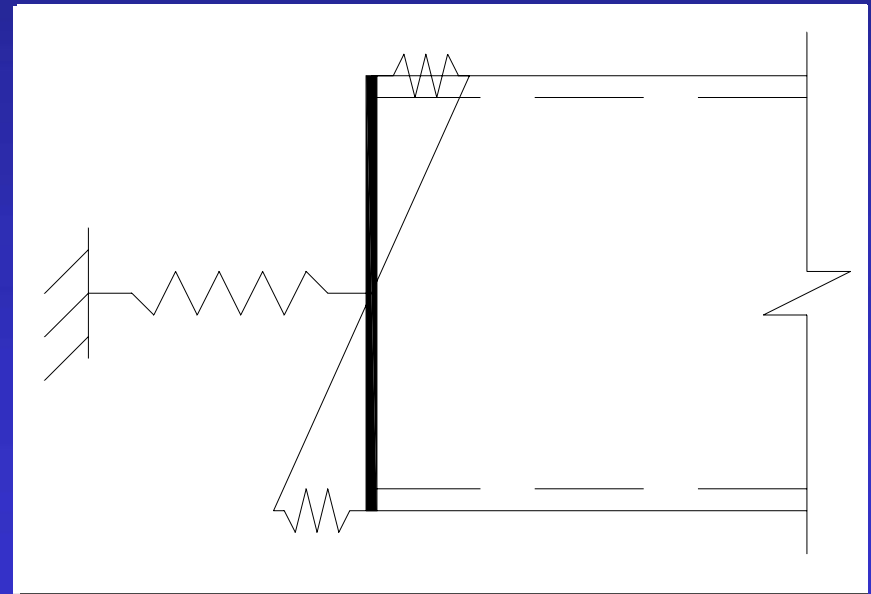


# Objectives

- Investigate the large deflection behaviour of steel beams at elevated temperatures
- Develop a simple hand calculation method for predicting the deflections and catenary forces in steel beams at elevated temperatures

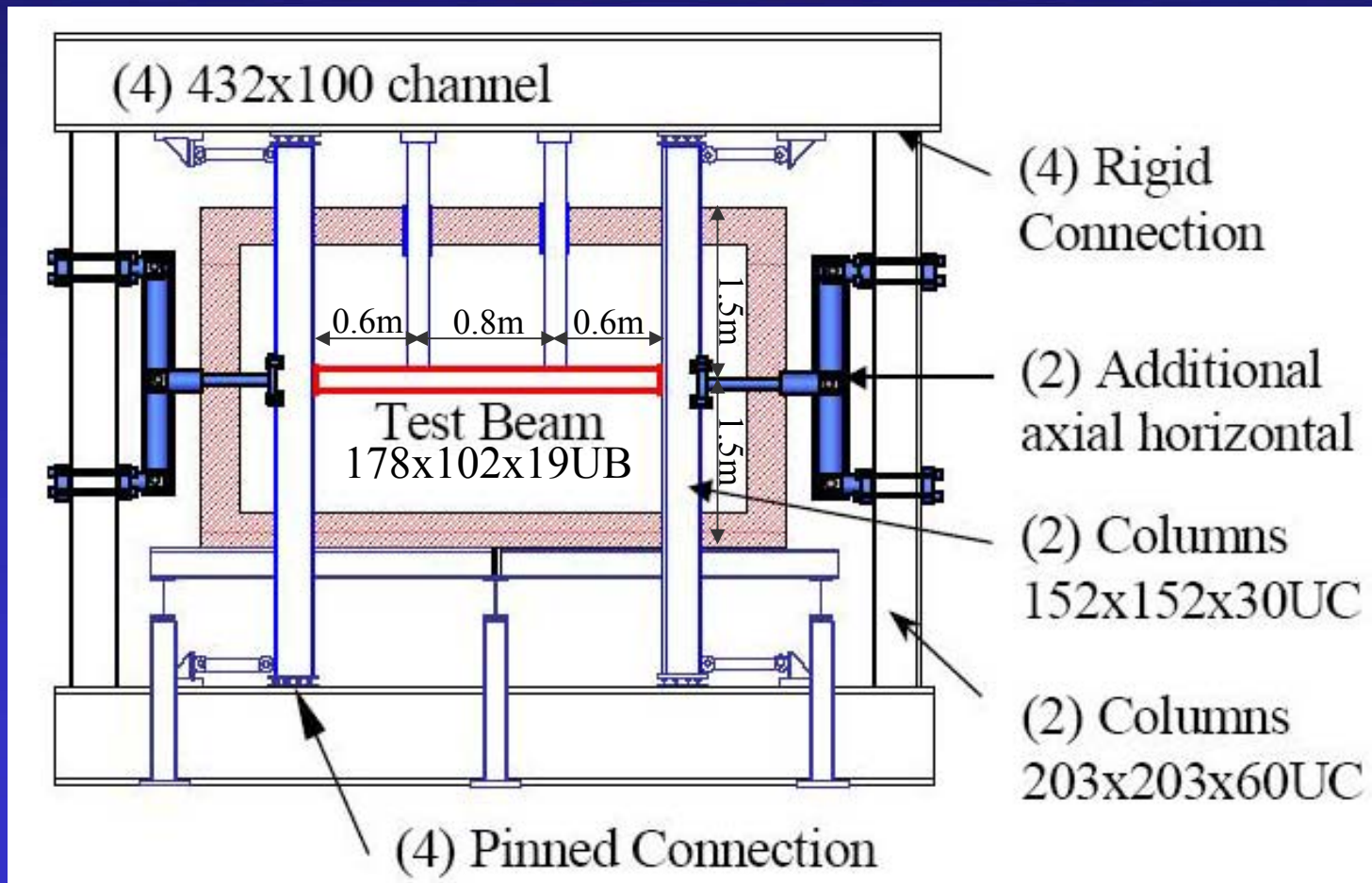
# Numerical Simulations by ABAQUS

- Shell element S4R
- Boundary conditions:
  - Stiff end plates applied to beam end sections
  - Spring elements applied at centre of beam end sections
  - Spring couple elements applied to the top and bottom flanges of beam end sections relative to section centre

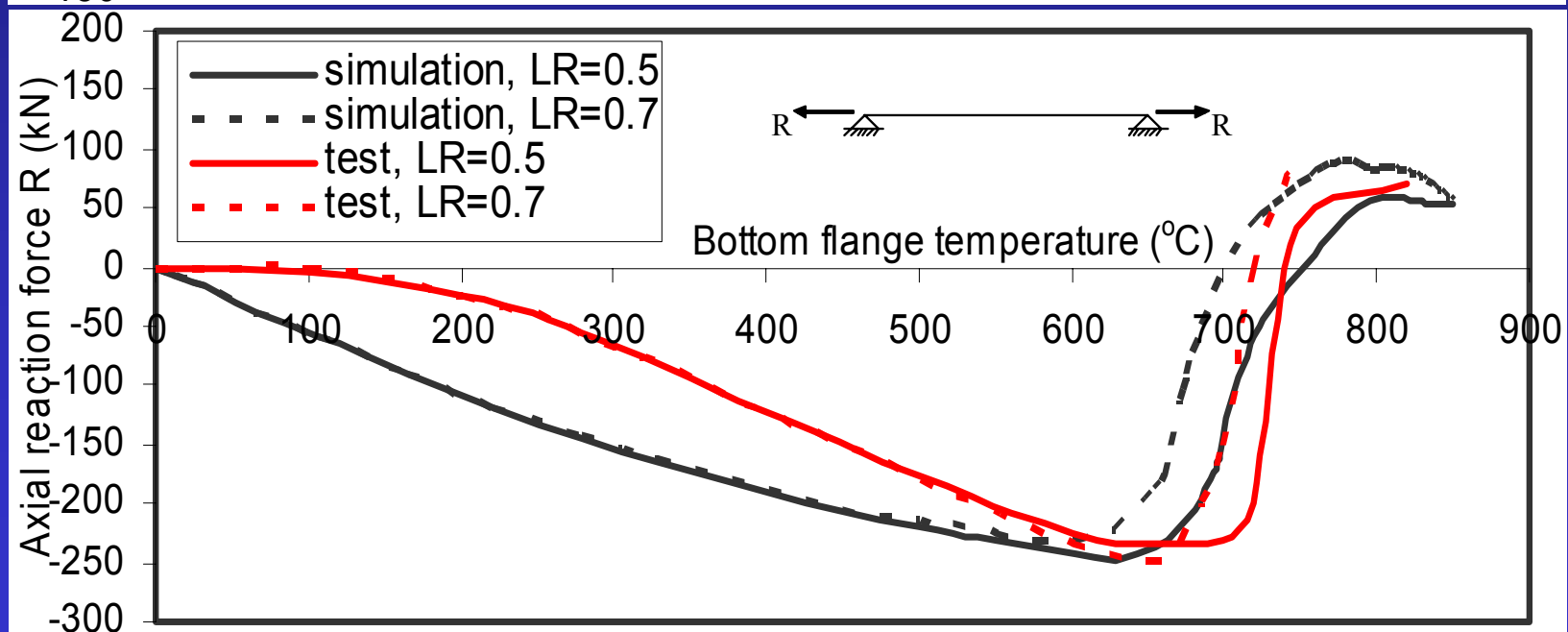
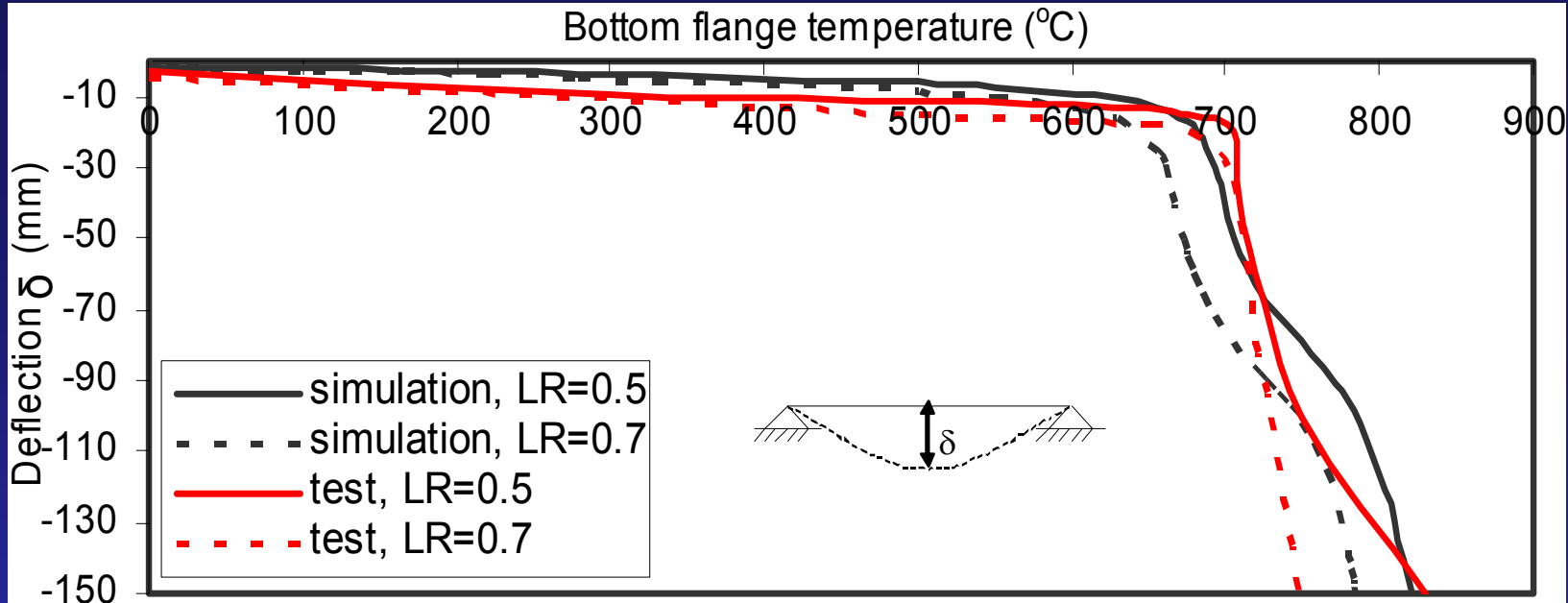


ABAQUS

# Model validation



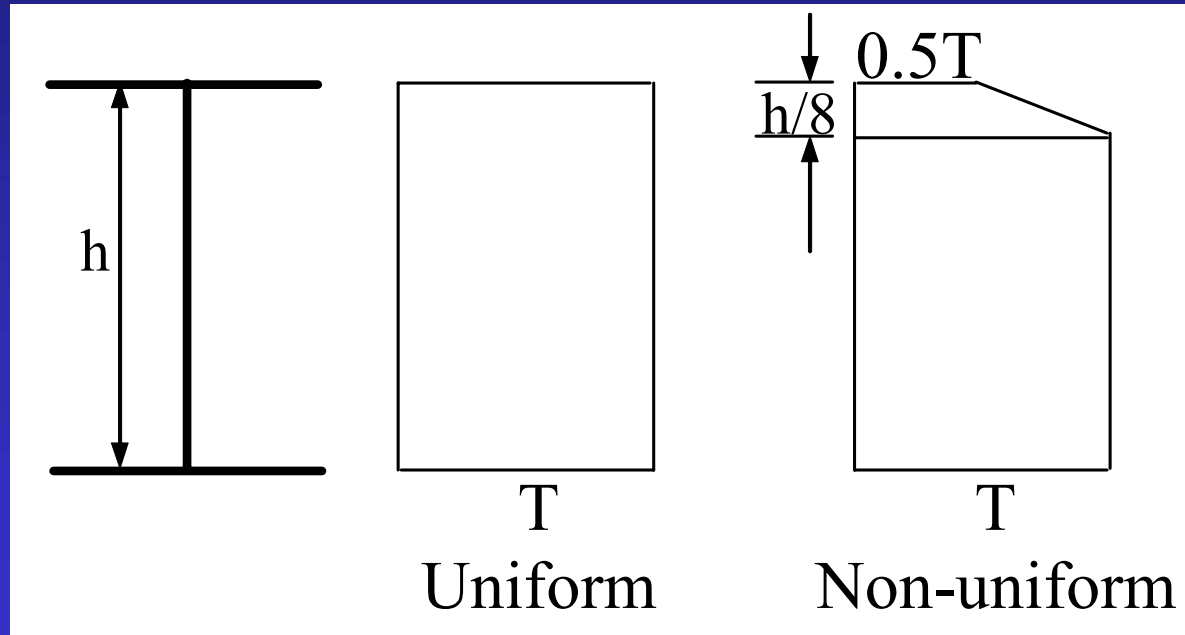
ABAQUS



ABAQUS

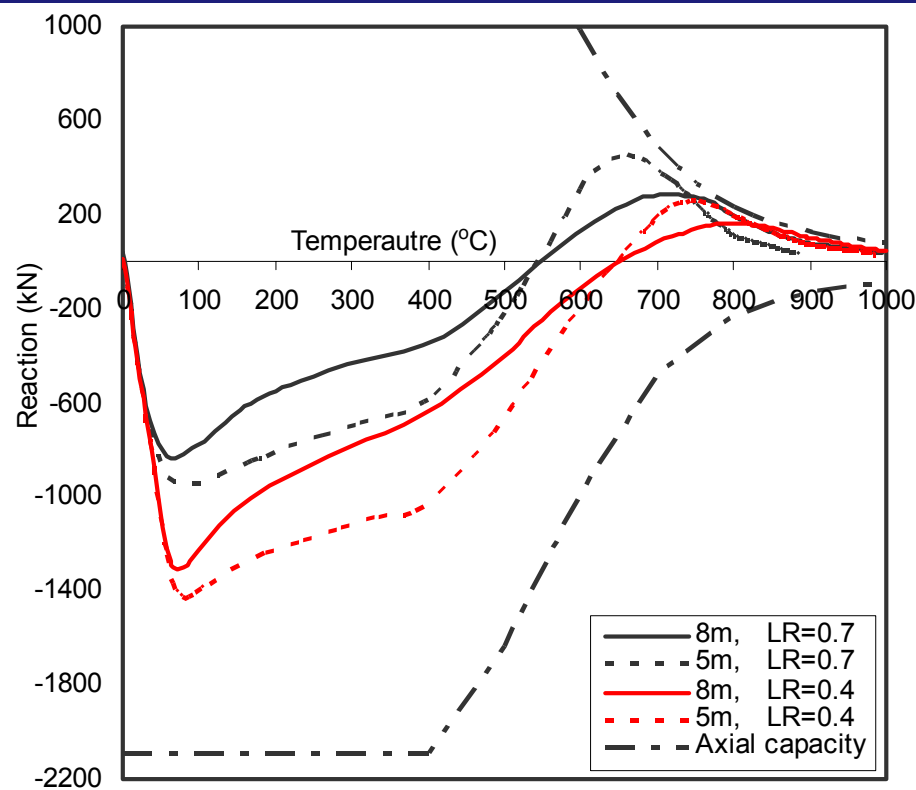
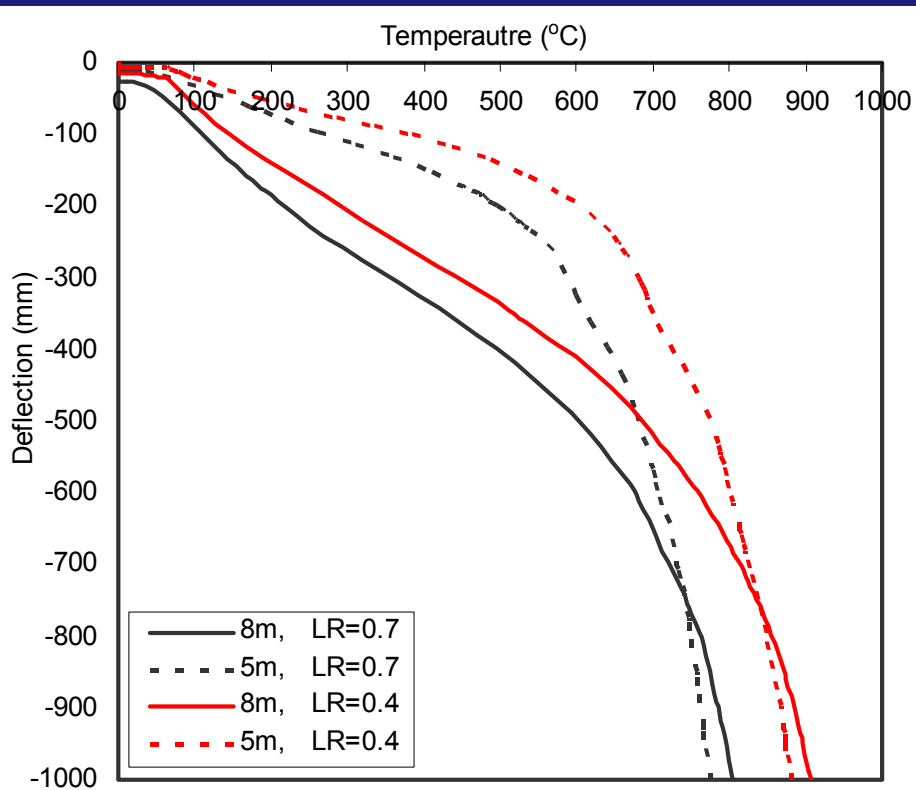
# Fully axially restrained beams with lateral restraints

## Temperature distributions



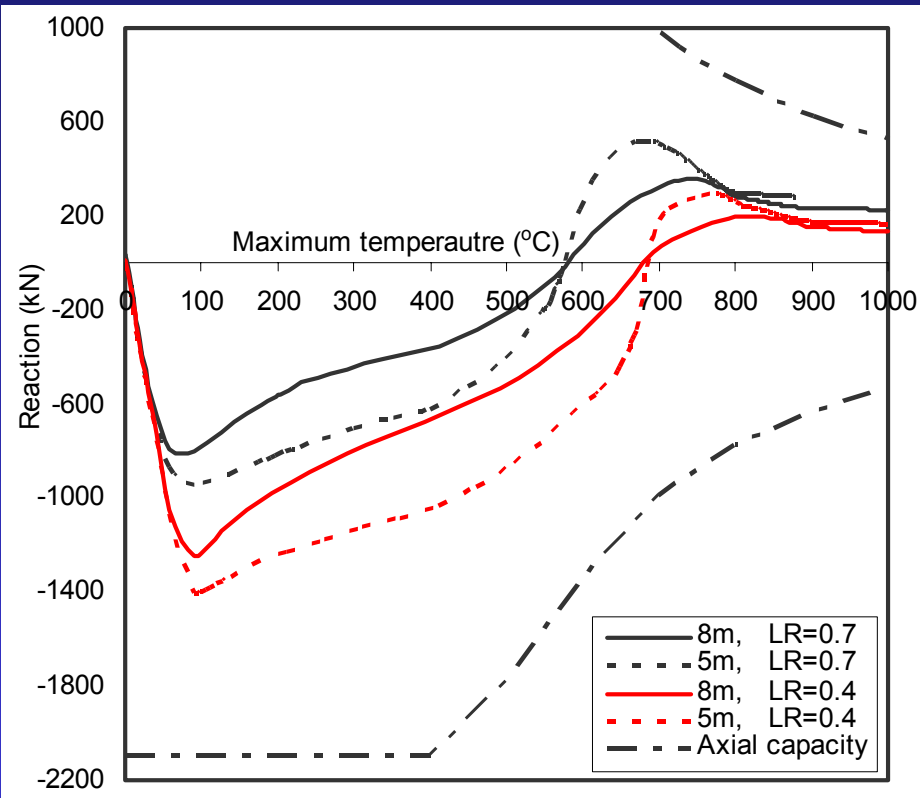
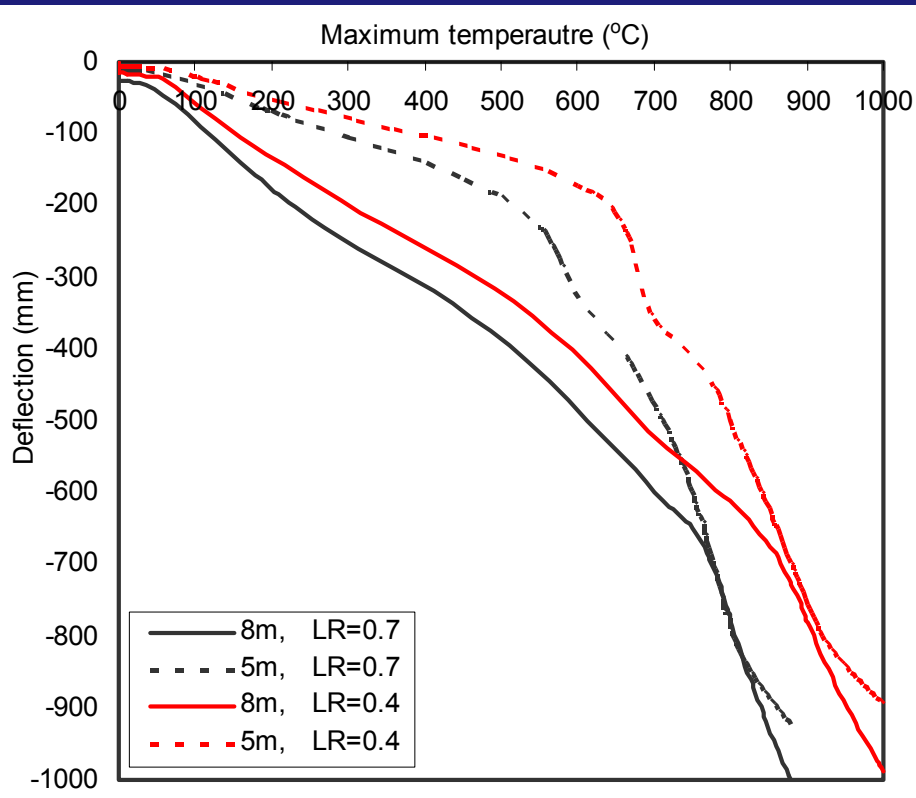
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## • Uniform temperature distribution



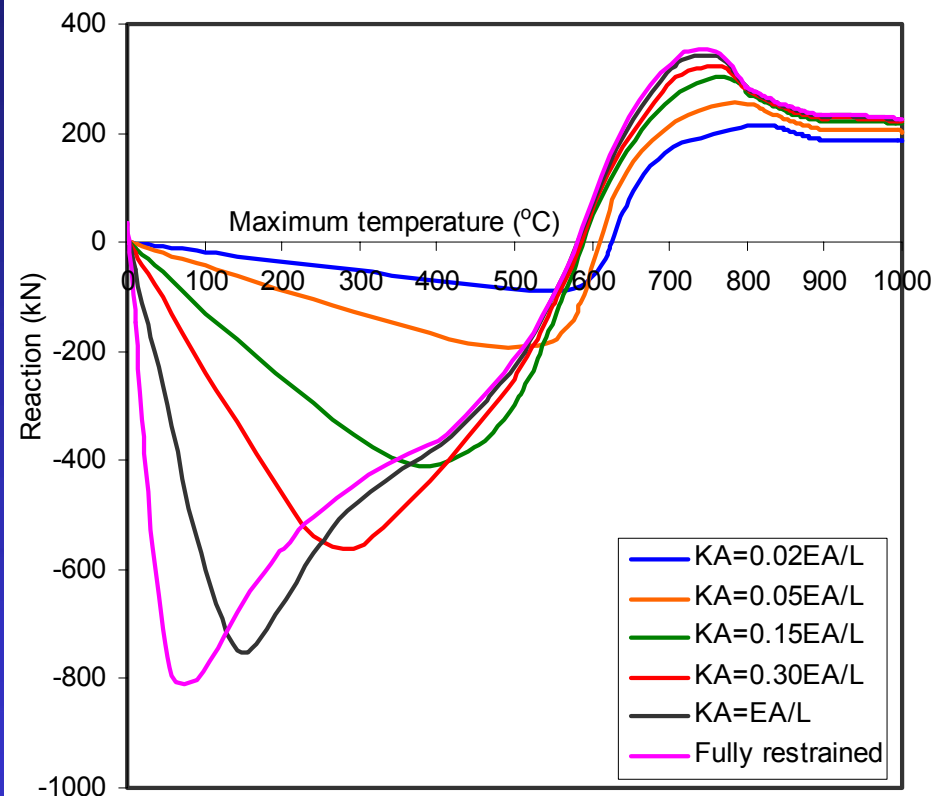
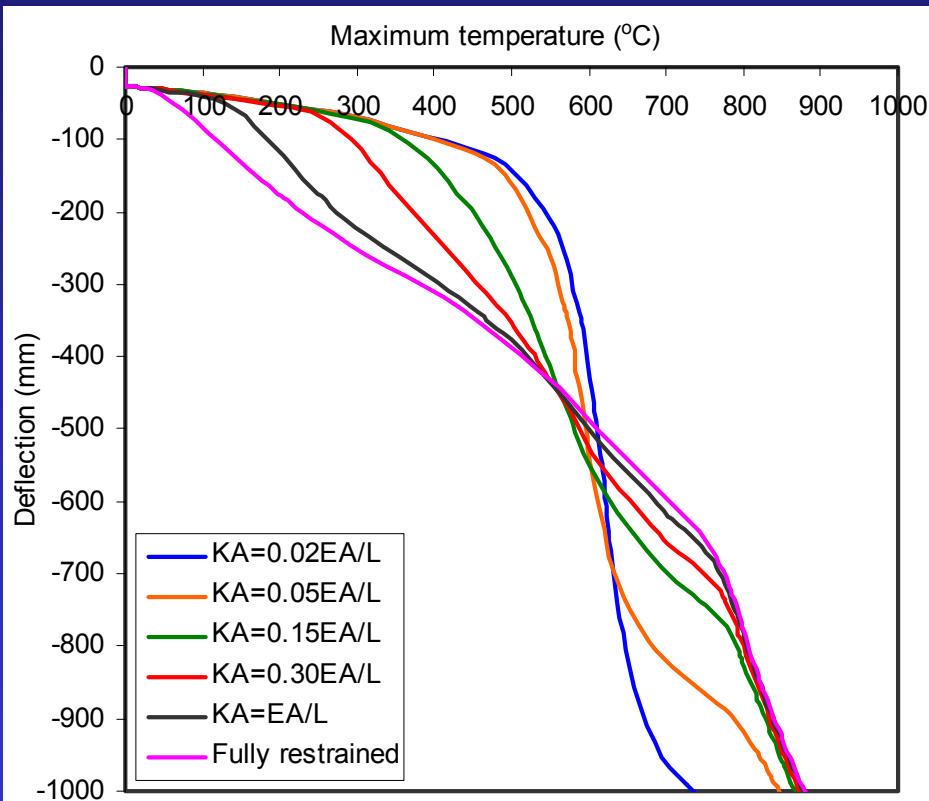
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## • Non-uniform temperature distribution



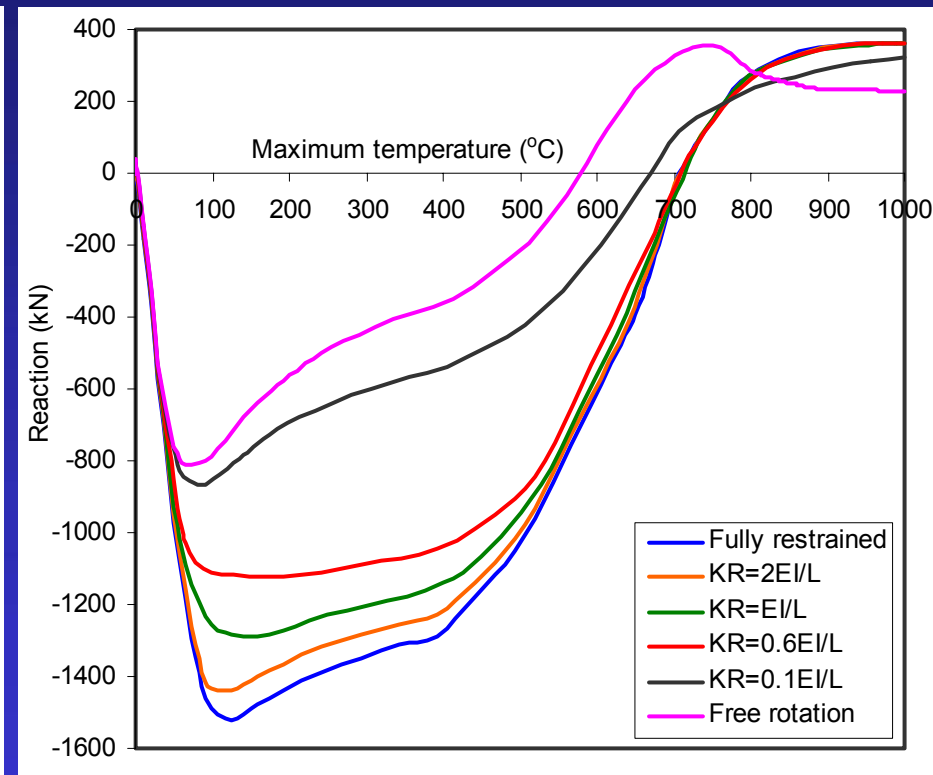
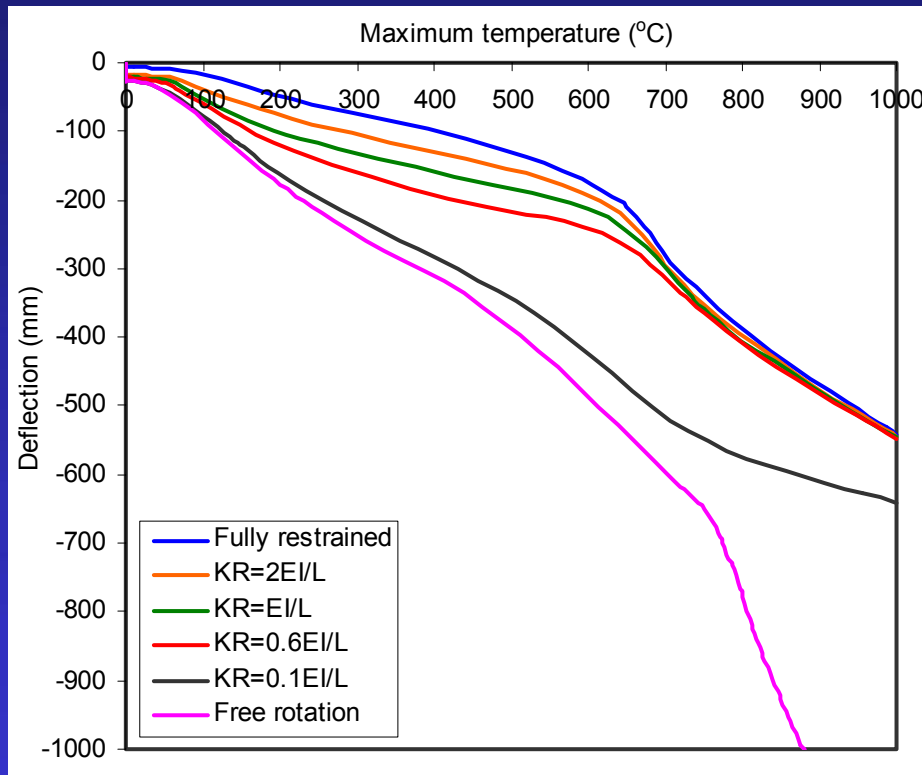
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# Laterally restrained beams with different levels of axial restraint



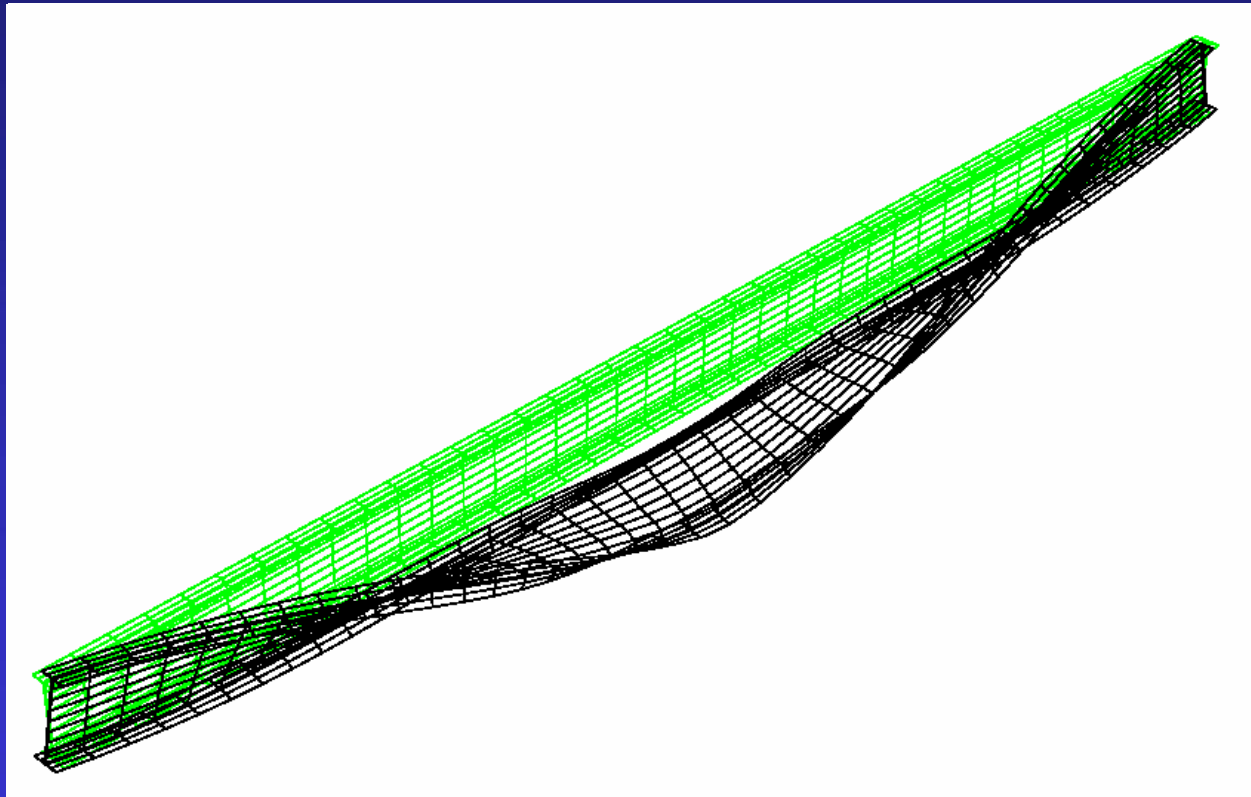
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# Laterally restrained beams with different levels of rotational restraint

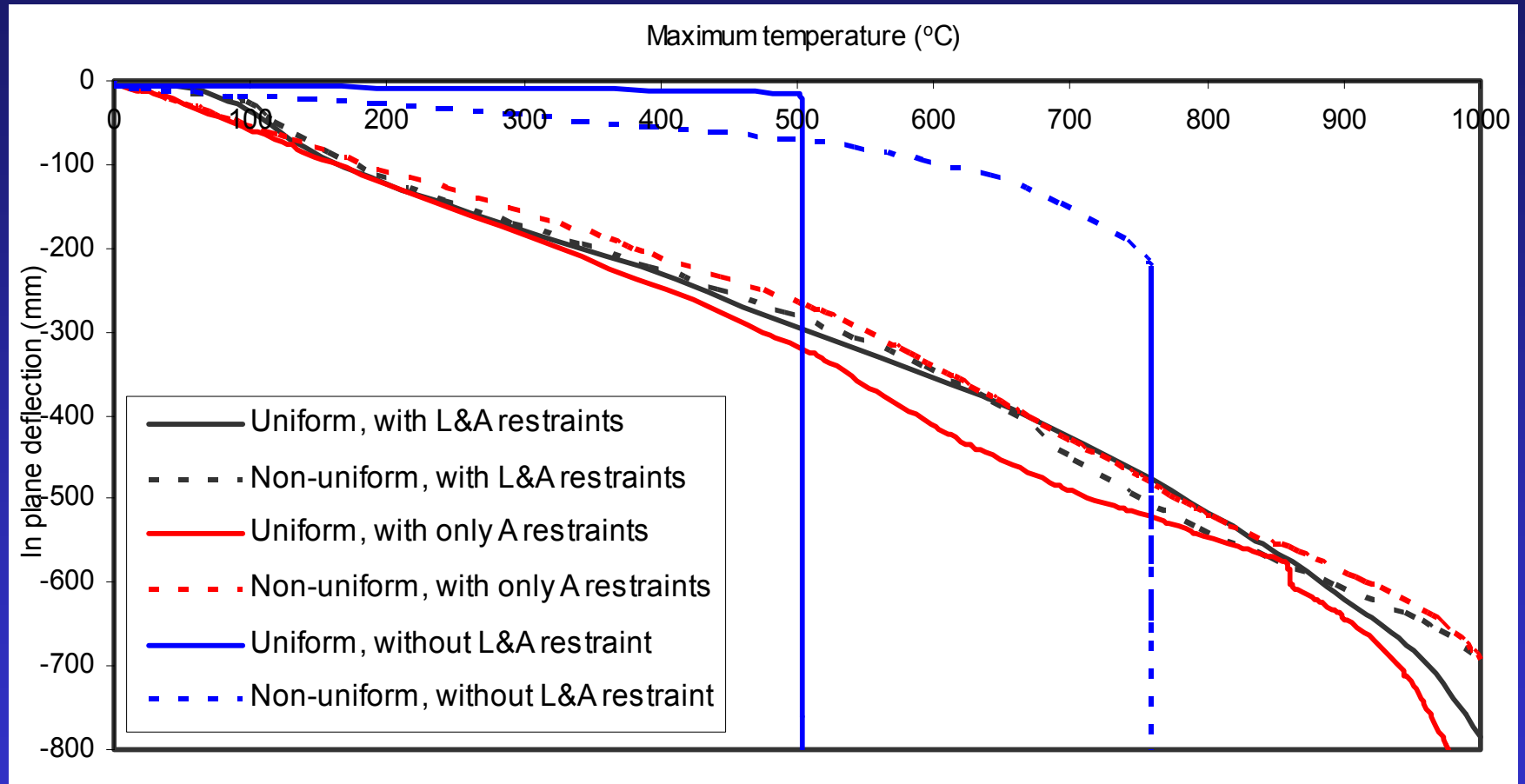


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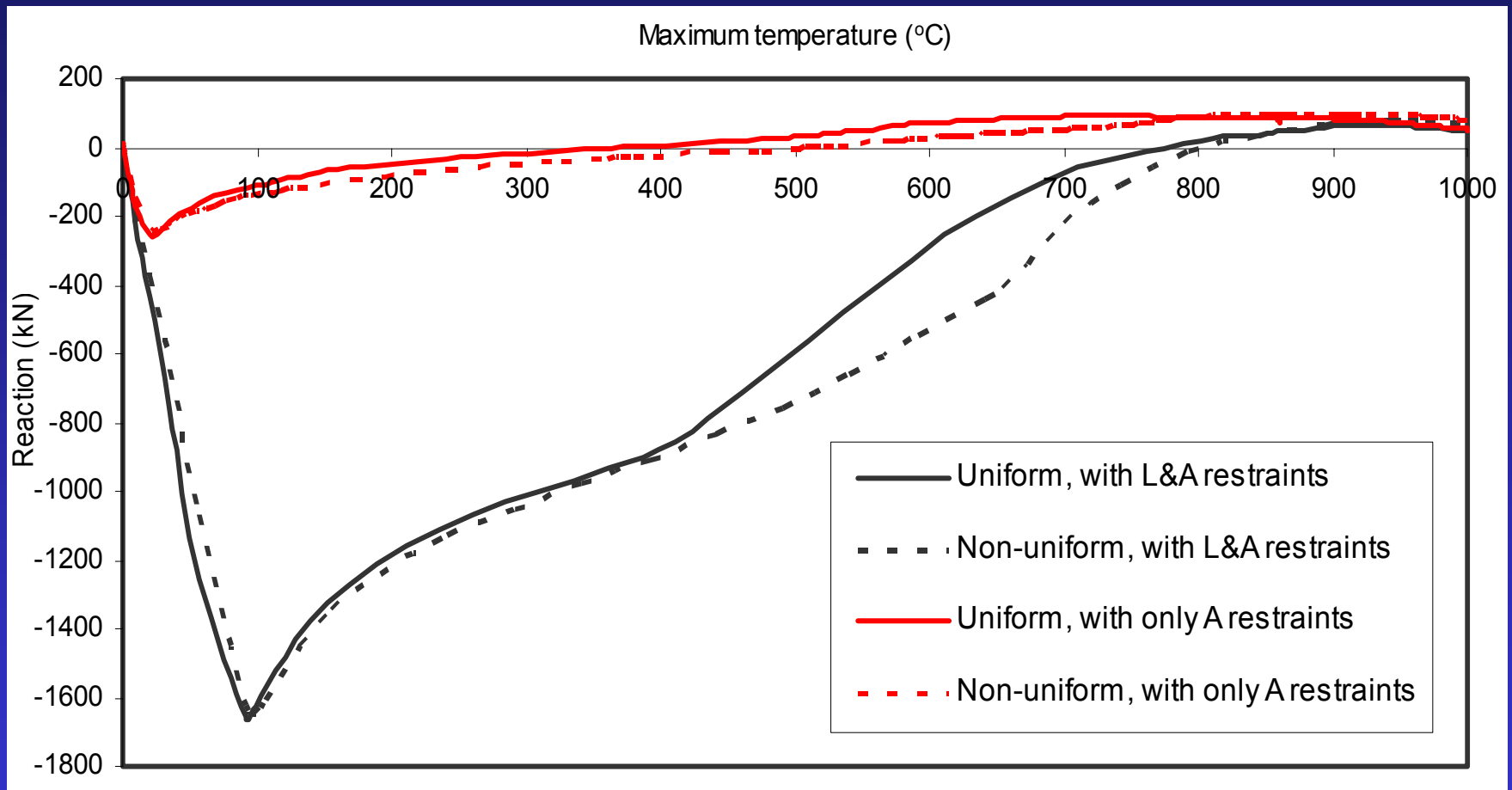
# Effect of lateral torsional buckling



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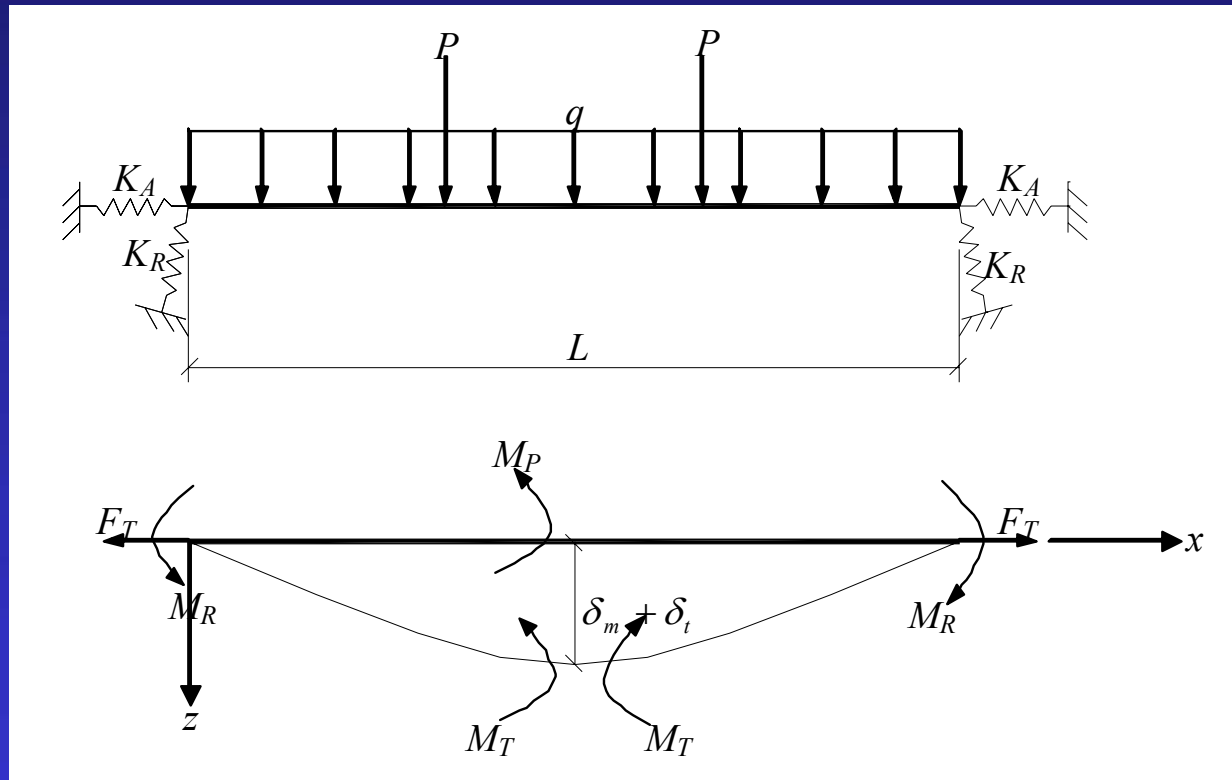
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# Hand Calculation Method



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# Equilibrium equations

$$F_T(\delta_m + \delta_t) + M_T + M_R + M_P = 0$$

$$F_T = K'_A \varepsilon_m = K'_A \frac{\Delta L_m}{L}$$

$$\frac{1}{K'_A} = \frac{1}{K_A} + \frac{L}{E_T A} + \frac{1}{K_A}$$

$$M_T = E_T I_y \varphi_m \Big|_{x=\frac{L}{2}}$$

$$M_R = K'_R \theta \Big|_{x=0}$$

$$\frac{1}{K'_R} = \frac{1}{K_R} + \frac{L}{E_T I_y} + \frac{1}{K_R}$$

$M_P$ : the externally applied free bending moment

# Deflection profiles

$$z(x) = z_m(x) + z_t(x)$$

$$\Delta L_m = \Delta L - \Delta L_t$$

$$\Delta L = \int_0^L \left[ 1 + \left( \frac{dz}{dx} \right)^2 \right]^{1/2} dx - L$$

$$\Delta L_t = \alpha TL$$

$$\varphi_m \Big|_{x=\frac{L}{2}} = \frac{d^2 z_m}{dx^2} \Big|_{x=\frac{L}{2}}$$

$$\theta \Big|_{x=0} = \frac{dz}{dx} \Big|_{x=0}$$

- Uniform temperature distribution

$$z_t = 0$$

- Zero end rotational restraint

Under UDL

$$z_m = \frac{16\delta_{m,\max}}{5L} \left( \frac{x^4}{L^3} - \frac{2x^3}{L^2} + x \right)$$

Under CPL

$z_m$  = free bend moment diagram

- Complete end rotational restraint

$$z_m = \frac{16\delta_{m,\max}}{L^2} \left( \frac{x^4}{L^2} - \frac{2x^3}{L} + x^2 \right)$$

- Non-uniform temperature distribution

- Zero end rotational restraint

$$z_t = -\frac{\alpha\Delta T}{2h}(x^2 - Lx)$$

Under UDL

$$z_m = z_{UDL,UT}$$

Under CPL

$$z_m = \frac{z_{UDL,UT} + z_{CPL,UT}}{2}$$

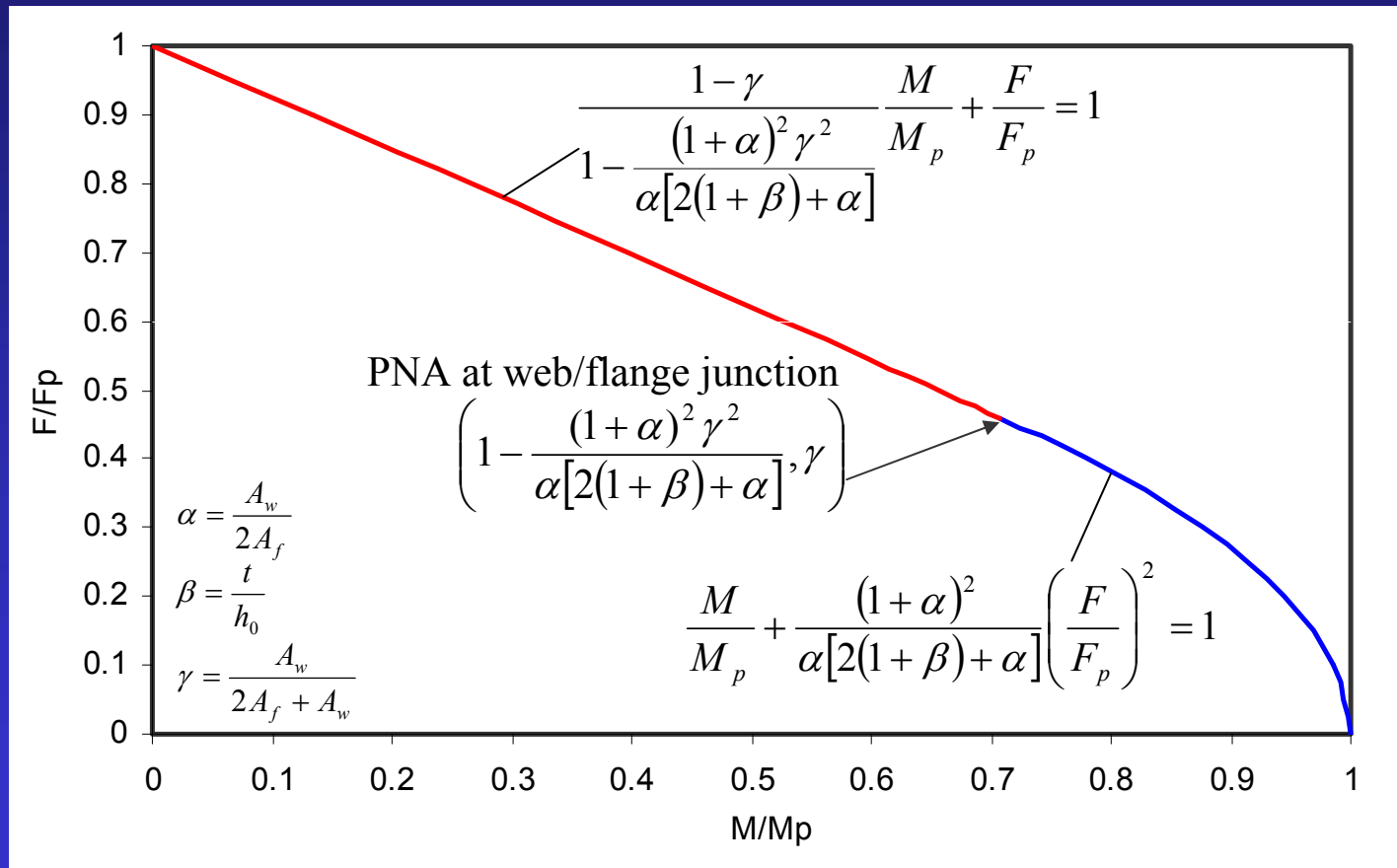
- Complete end rotational restraint

$$z_t = 0$$

$$z_m = \frac{16\delta_{m,\max}}{L^2} \left( \frac{x^4}{L^2} - \frac{2x^3}{L} + x^2 \right)$$

$$M_t = \frac{E_T I_y \alpha \Delta T}{h}$$

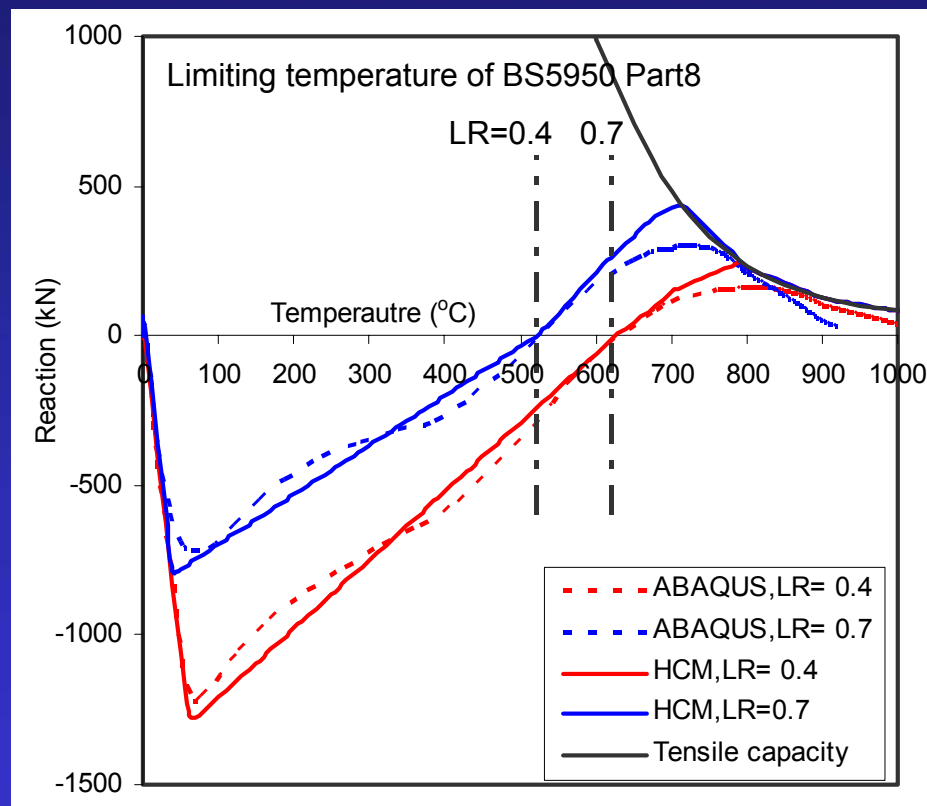
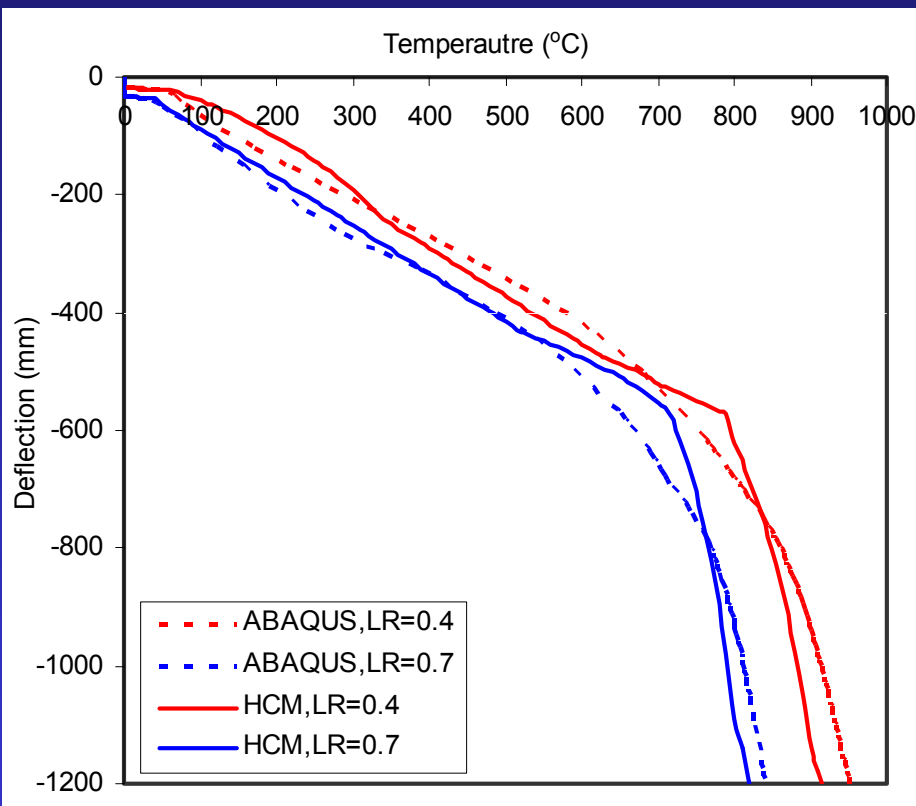
# Axial load & bending moment interaction



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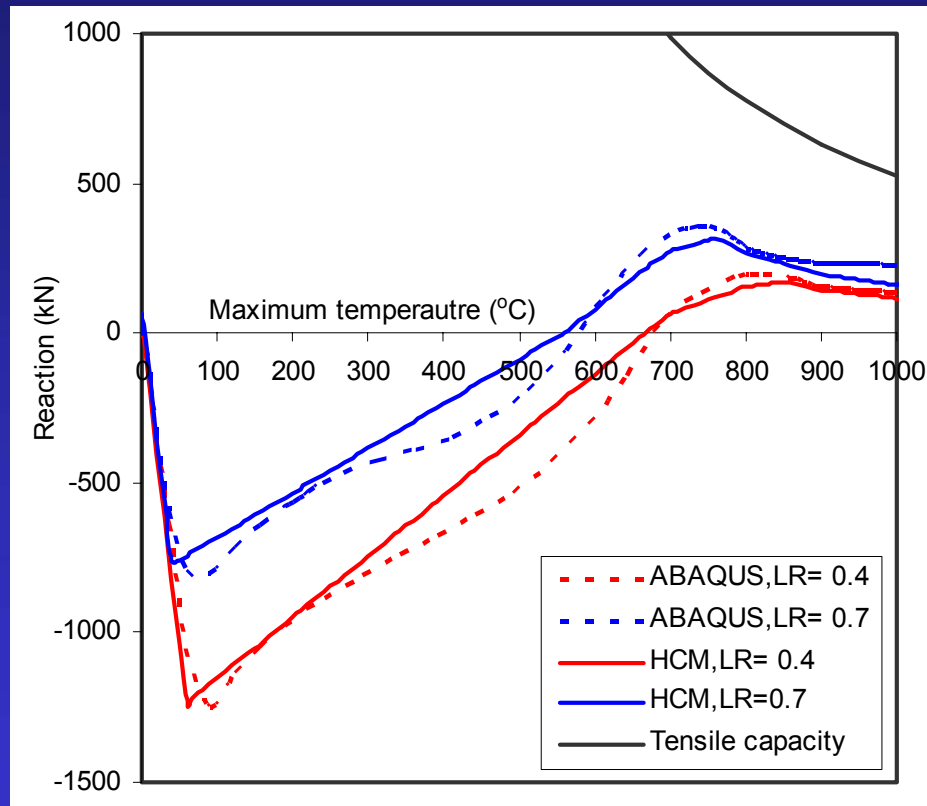
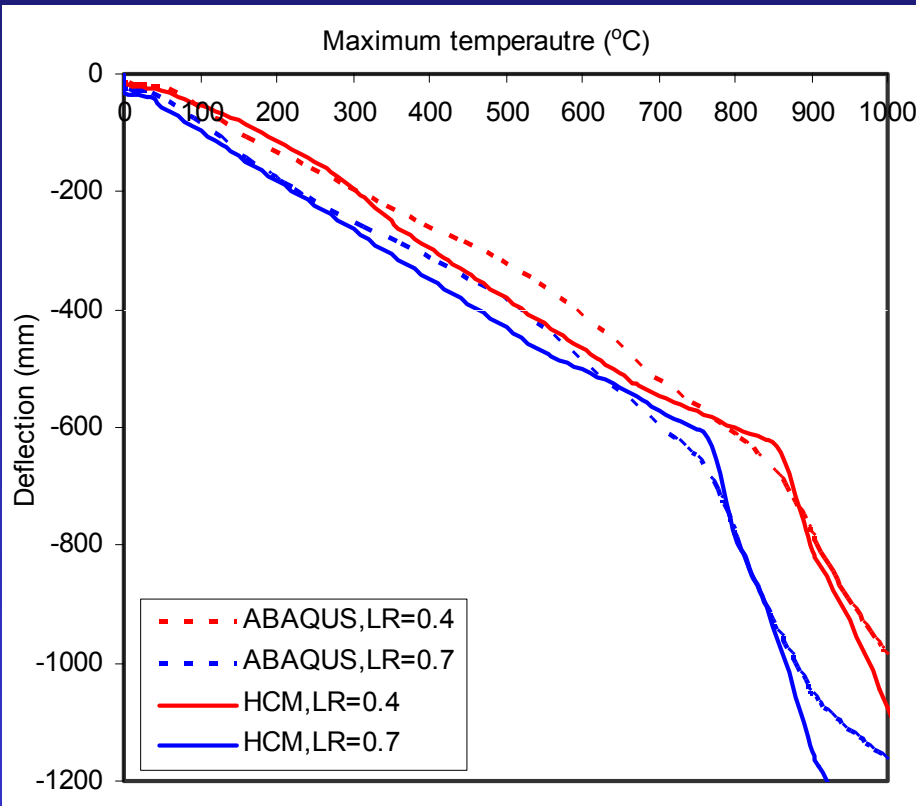
# Validation

- Complete axial restraint, zero rotational restraint  
uniform temperature, UDL



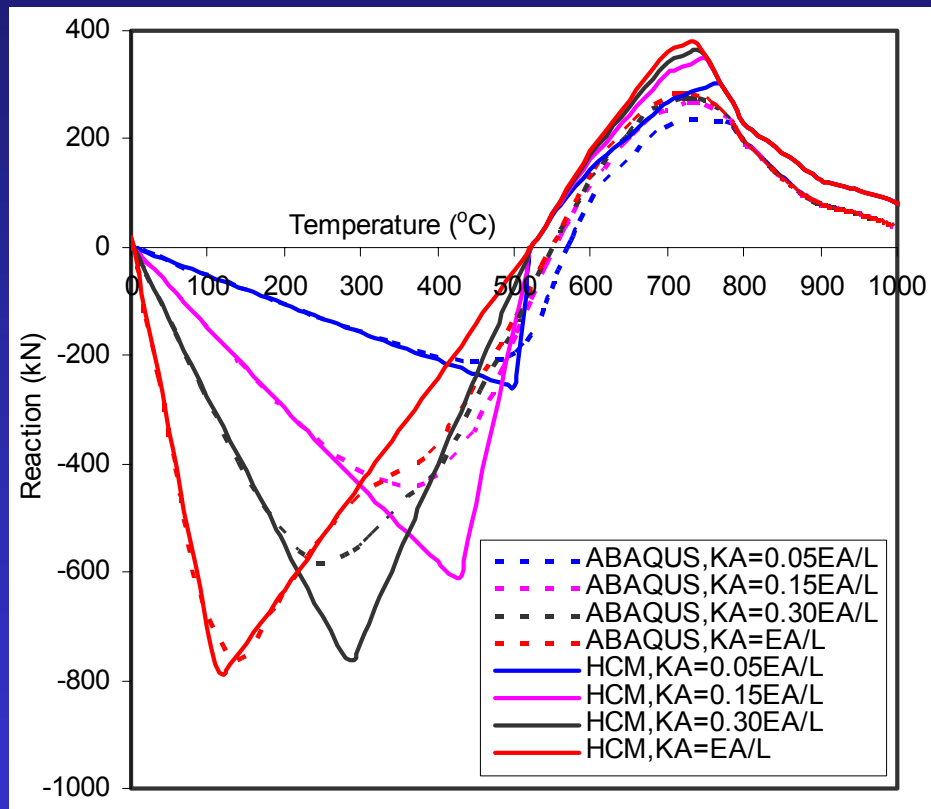
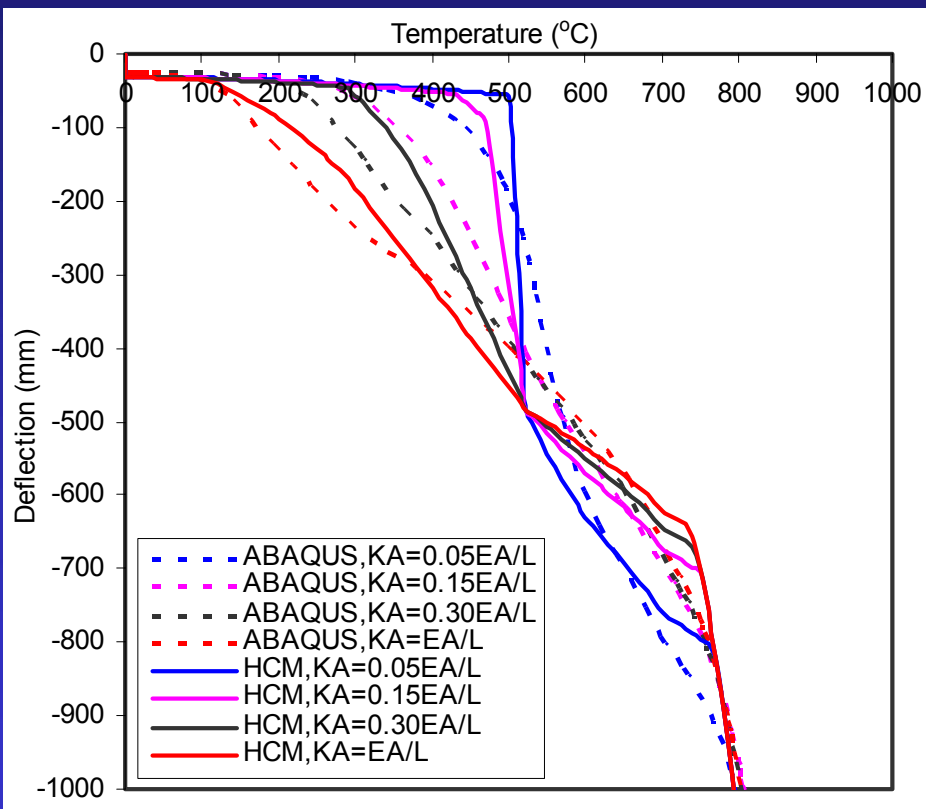
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- Complete axial restraint, zero rotational restraint  
non-uniform temperature, central point load



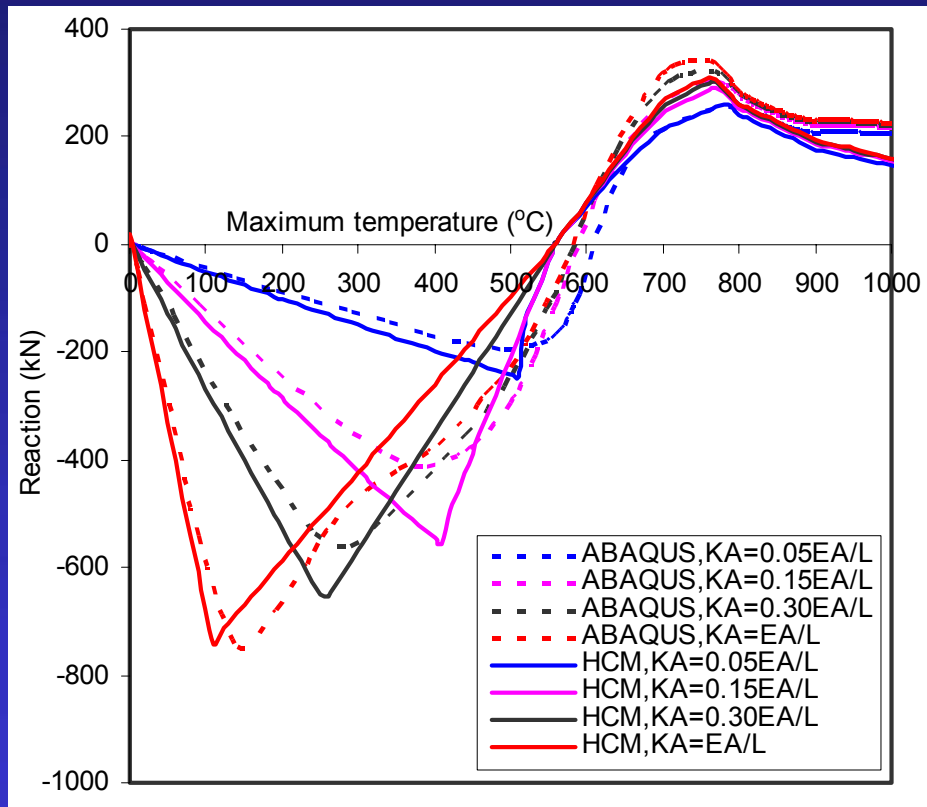
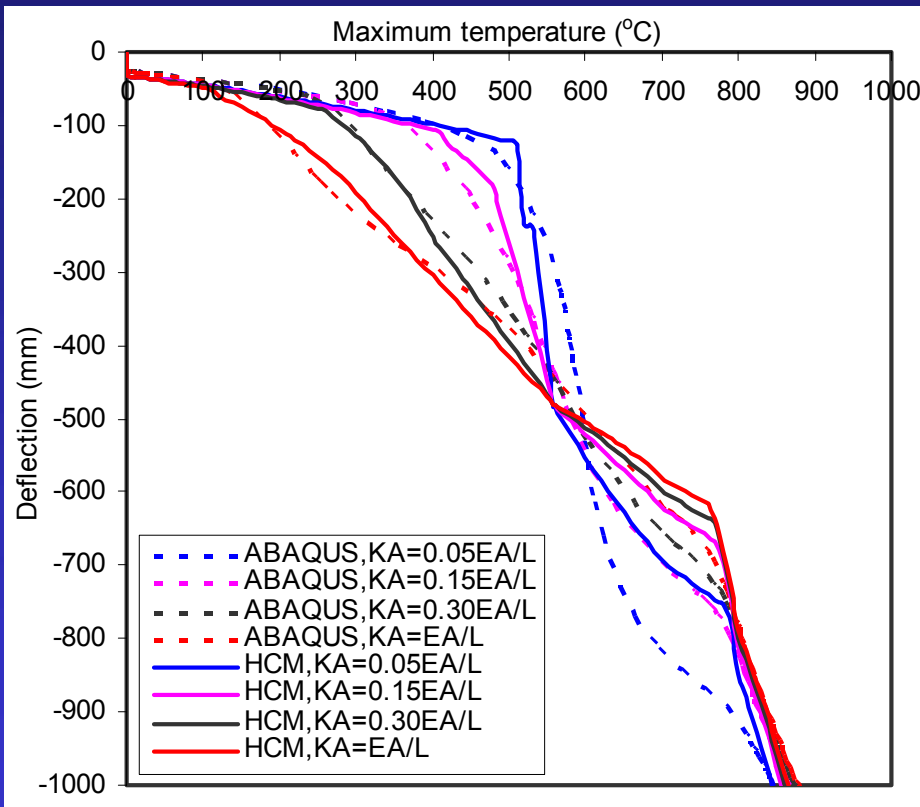
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- Different levels of axial restraint, zero rotational restraint  
uniform temperature, central point load, load ratio = 0.7



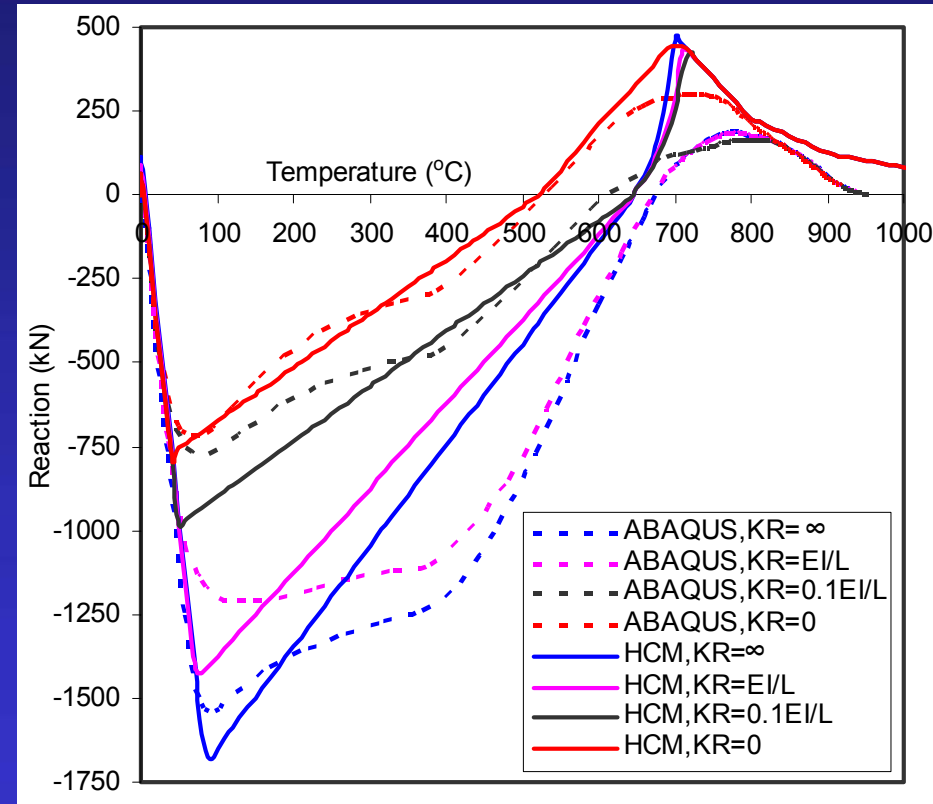
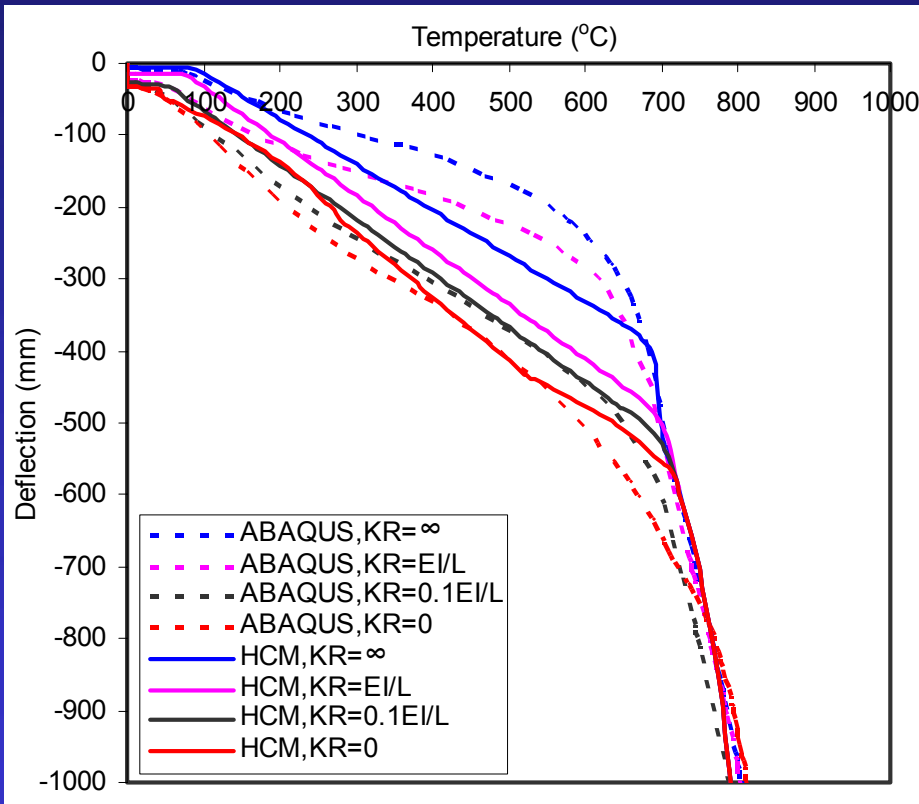
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- Different levels of axial restraint, zero rotational restraint  
non-uniform temperature, central point load, load ratio = 0.7



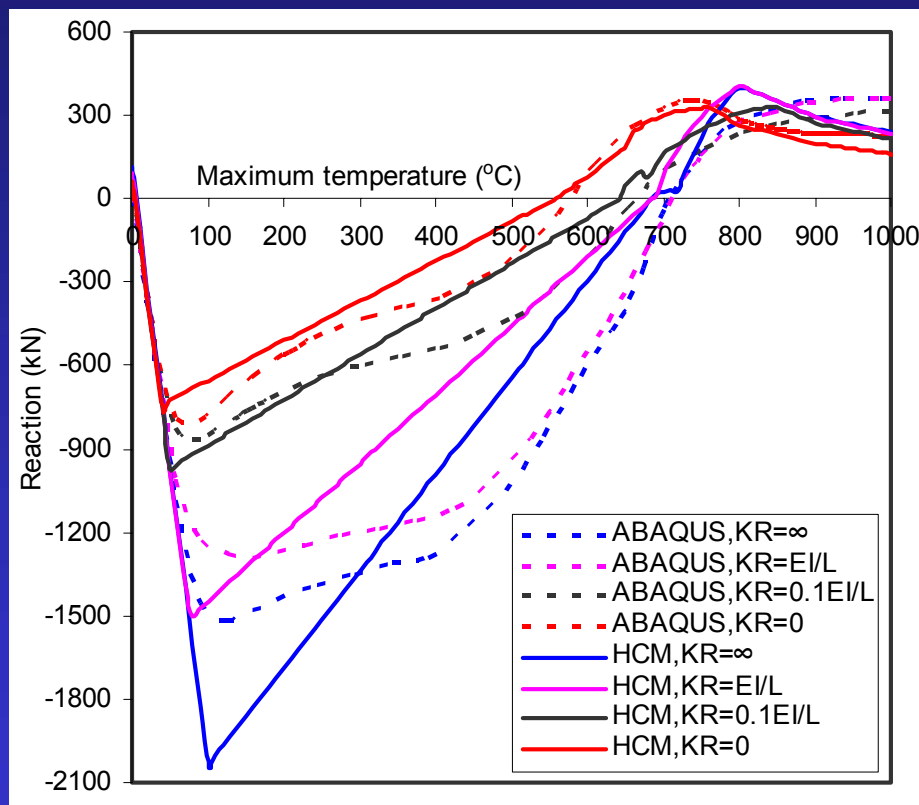
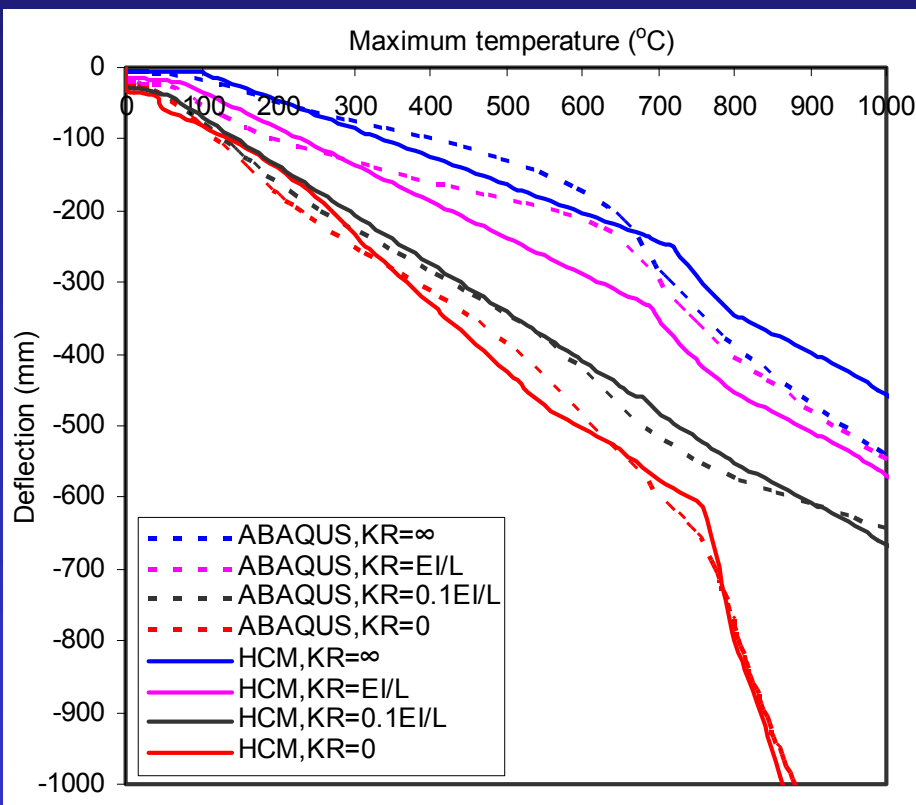
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- Different levels of rotational restraint, complete axial restraint  
uniform temperature, UDL, load ratio = 0.7



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- Different levels of rotational restraint, complete axial restraint  
uniform temperature, central point load, load ratio = 0.7



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# Conclusions

- If a steel beam is reliably provided with some axial restraints, catenary action will occur and will enable the beam to survive very high temperatures without a collapse
- Whether a beam will experience lateral torsional buckling or not will only have some minor effects on its large deflection behaviour
- A simplified hand calculation method is developed to predict the maximum deflection and catenary force in a steel beam, which can be used in design applications

**Thank you!**